

Project: Land at Foxhole Farm, Bolney

Report Title: **Drainage Technical Note**

Document Ref.: 598-003-001

Revision: B

Date: 22/07/2026

Revision Status

Rev	Amendments	Date	Author	Checked	Approved
A	Initial Issue	22-07-2026	TZ	MK	MK
B	Orifice and Flow Controls details added	24-07-2026	TZ	MK	MK

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1 Planning Policy Context

1.1 Executive Summary

- I. This Drainage Technical note has been produced in accordance with National and Local Planning Policy on behalf of Cala Homes (South Homes Counties) to support the Reserved Matters Application for the erection of up to 200 dwellings; a community building encompassing land for education provision, together with associated access, ancillary parking and landscaping.
- II. The development requires site-specific surface water and foul sewer drainage infrastructure must complete with the requirements of the National Planning Policy Framework (NPPF), which mandates the use of Sustainable Drainage Systems (SuDS) for major developments where feasible.
- III. The Lead Local Flood Authority strongly advocates for the integration of SuDS within new developments. All new drainage systems will be designed in accordance with the following guidelines and standards:
 - Planning Practice Guidance (PPG)
 - Non-Statutory Technical Standards for Sustainable Drainage Systems
 - Building Regulations – Approved Document H (Drainage and Waste Disposal)
 - Local authority sustainable drainage guidance
 - Latest version of Design Construction Guidance
- IV. This report is to be read in conjunction with all planning, architectural and other reports that will accompany the planning application.
- V. From the information available this report confirms that a compliant sustainable drainage system design can be provided, which will discharge surface water in accordance with National Planning Practice Guidance (NPPG) and the CIRIA SuDS hierarchy while providing storage for all storm return periods up to and including the climate change adjusted 1:100-year rainfall event.
- VI. This report concludes that the site is not at risk of flooding from tidal or fluvial sources, overland flows, or groundwater.
- VII. In terms of foul and surface water drainage the proposed development is suitable at this location.

1.2 West Sussex County Council:

- 1.2.1 West Sussex County Council (WSCC) published local guidance pertaining to Sustainable Drainage Systems (SuDS). This report outlines the key requirements for infiltration as specified in the WSCC guidance.
- 1.2.2 The WSCC guidance stipulates the following criteria for onsite storage and restricted discharge.
- Greenfield Runoff Rate: Discharge rates must be calculated using the Greenfield runoff method, considering only positively drained impermeable areas. The proposed discharge rate should closely mimic the Qbar value obtained from these calculations.
 - Half-Drain Down Time: The system must be designed to ensure that half of the stored volume drains within 24 hours after a 1 in 30-year rainfall event, inclusive of climate change allowances.
 - Water Quality Consideration: The design must incorporate water quality improvements following the principles set out in the CIRIA SuDS Manual (Report C753).
 - First Flush Capture: The first 5mm of any rainfall event should be captured and managed on-site to prevent contamination of receiving water bodies.
- 1.2.3 According to WSCC SuDS guidance, post-development runoff rates for greenfield sites must be restricted to equivalent greenfield runoff rates. A staged approach or QBAR rural methodology should be utilised.
- 1.2.4 The following surface water strategy has been developed in accordance with local policy and SuDS requirements, ensuring compliance and effective management of surface water.
- 1.2.5 Adhering to these guidelines will ensure that the proposed development meets the necessary standards for sustainable drainage, promoting environmental sustainability and regulatory compliance.

1.3 Review of the Flood Risk Assessment and Drainage Strategy report prepared by others:

- 1.3.1 Both Flood Risk Assessment and Drainage Strategy reports (ref; 1620011691-012) have been prepared by Ramboll in April 2025 to support the outline planning application.
- 1.3.2 Flood Risk Assessment identified areas at risk of surface water flooding. No overland flow paths have been indicated to be leading onto the development site.
- 1.3.3 FRA report states that: *'A potential flow path is indicated to be present along the course of the ordinary watercourse leading away from the site in the northwest. This includes the area either side of the culvert beneath Foxhole Lane and extends into part of the site. The flow path is shown to cease when it reaches the course of the Rout Gill to the west of the site as it flows approximately north to south. Potential flow paths are also indicated in the Mapping along The Street to the east of the site and along Cowfold Road to the south. Adjacent to the south of the site, a potential flow path is shown to overlap with the ditch that is oriented approximately east to west and is located adjacent to the southern boundary. In addition to the potential flow paths, an area of High risk is indicated in the northeast corner of the site. This is likely to be the result of lower topography in this area which is indicated on the site topographical survey and it aligns with observations made onsite of saturated ground and pooling in this area (and lower topography).'*
- 1.3.4 All areas identified as being at risk of surface water flooding are located within the lower-lying parts of the site. The proposed site layout has been designed so that no dwellings are located within these areas.
- 1.3.5 The proposed surface water drainage strategy has been developed to ensure that runoff from landscaped areas and all contributing catchments is included within the drainage

calculations. Surface water runoff from the higher ground will be intercepted by cut-off swales and attenuation basins. The collected runoff will then be conveyed to detention basins before being discharged, via the proposed on-site drainage network, at a restricted rate to the existing watercourse located to the south of the proposed development site.

- 1.3.6 The entire surface water drainage network has been modelled to ensure that potential surface water runoff is effectively collected, conveyed, and discharged through the site in a controlled manner. This approach reduces runoff rates to greenfield-equivalent discharge where applicable and prevents surface water flooding within the proposed development and on adjacent land.
- 1.3.7 The proposed drainage design has been developed in accordance with the approved planning drainage strategy. The design takes into account surface water runoff from the higher areas of the site, which will be intercepted by the proposed on-site drainage system. The development has been divided into two drainage catchments—a northern parcel and a southern parcel—each with a separate surface water discharge location.
- 1.3.8 Surface water runoff from the northern parcel is proposed to discharge to the existing culvert beneath Foxhole Lane, located to the west of the development site. Runoff from the southern parcel is proposed to discharge to the existing ditch running generally east to west along Cowfold Road and the southern boundary of the site, before subsequently discharging into the existing sewer network within Cowfold Road.
- 1.3.9 Copy of both FRA and Drainage Statement can be found in Appendix A at the rear of this statement.

2 Existing Site

- 2.1.1 The proposed development is located at Foxhole Farm, Bolney, on land to the east of Foxhole Lane (postcode: RH17 5NB) at Ordnance Survey grid reference: TQ 25855 22825. The development comprises two parcels, referred to as the Northern Parcel and the Southern Parcel.
- 2.1.2 The site is bounded by woodland, open green space, and residential dwellings to the north. To the east lie The Street and additional residential properties. The A272 Cowfold Road forms the southern boundary, while Foxhole Lane and agricultural land are located to the west.



Figure 1. Site Location Plan

2.2 Site Description

- 2.2.1 The development site is approximately 16.9Ha in area. The site currently consists of greenfield land, existing trees and farm buildings.
- 2.2.2 The topography of the proposed Northern Parcel indicates that the site generally falls from the south-east towards the west. Existing ground levels range from approximately 42.0mAOD in the south-east to approximately 30.1mAOD along the western boundary. The topographical survey also indicates localised falls towards the north, east, and west across the site.
- 2.2.3 The proposed Southern Parcel generally falls from north to south. The existing topographical survey indicates that ground levels range from approximately 34.3–35.0mAOD at the northern end of the site, falling to approximately 22.0mAOD at the southern boundary adjacent to the A272 Cowfold Road.
- 2.2.4 A copy of the topographical survey can be found in Appendix B at the rear of this report.

2.3 Existing Drainage

- 2.3.1 The existing site comprises undeveloped greenfield land with no impermeable surfaces and, therefore, no positively drained areas.
- 2.3.2 Based on the topographical survey and a review of the previous Drainage Strategy and Flood Risk Assessment (FRA) reports, surface water runoff from the north-western part of the site is understood to drain towards an existing culvert, which conveys flows beneath Foxhole Lane before discharging into the existing watercourse to the west of the site.
- 2.3.3 The southern part of the site currently drains towards the existing ditch located along the southern boundary. The existing ditch subsequently discharges into the existing surface water sewer network within Cowfold Road.
- 2.3.4 Under existing conditions, rainfall drains as overland greenfield runoff in accordance with the site's natural topography. This results in the ponding of surface water in the lower-lying areas of the site, which generally corresponds with the areas identified as being at risk of surface water flooding on the Environment Agency mapping.
- 2.3.5 The pre-development greenfield runoff rates have been calculated based on the proposed positively drained impermeable areas together with the contributing landscaped areas that will be intercepted by the proposed on-site drainage system.
- 2.3.6 The landscaped areas have been included within the onsite drainage calculations. The runoff coefficient (Cv) of 0.4 has been adopted for the landscaped areas to represent the greenfield runoff characteristics of these catchments. This provides a more realistic representation of existing runoff conditions and enables more accurate surface water drainage calculations.

2.4 Greenfield runoff calculations

- 2.4.1 The pre-development greenfield runoff rates have been calculated using the HR Wallingford Greenfield Runoff Tool and the updated FEH methodology. The calculated greenfield runoff rate for the site is 5.5 l/s/ha.
- 2.4.2 The total contributing area for the Northern Parcel, including the landscaped areas that will drain to the proposed on-site drainage system via cut-off swales and filter drains, is 70,315m² (7.03ha). Based on the calculated greenfield runoff rate, the proposed surface water discharge rate for the Northern Parcel will be restricted to 38.7l/s.
- 2.4.3 The total contributing area for the Southern Parcel, including the landscaped areas that will drain to the proposed on-site drainage system via cut-off swales and filter drains, is 84,445m² (8.44ha). Based on the calculated greenfield runoff rate, the proposed surface water discharge rate for the Southern Parcel will be restricted to 46.4l/s.
- 2.4.4 The pre-development greenfield runoff rate calculation results and area plan are provided in Appendix C at the rear of this statement.

3 Site Geology

3.1 Ground Conditions

- 3.1.1 An intrusive ground investigation was undertaken by Geo-Environmental to establish the site's ground conditions and assess the feasibility of infiltration drainage. The investigation comprised eight dynamic sampler boreholes incorporating Standard Penetration Tests (SPTs) to a maximum depth of 4.0 m below ground level (m bgl). In addition, eight groundwater monitoring wells were installed to a depth of 4.0 m bgl, with groundwater levels monitored on a further six occasions between November 2023 and April 2024 to establish seasonal groundwater conditions.
- 3.1.2 The ground investigation confirmed that the site is generally underlain by topsoil over the Upper Tunbridge Wells Formation. It was noted that ground conditions in the northern part of the site comprise the Upper Tunbridge Wells Formation, rather than the anticipated Weald Clay Formation, as indicated by the published geological mapping.
- 3.1.3 An extract from the Ground Investigation Report, summarising the generalised ground conditions encountered across the site, is provided in the table below.

Top (m bgl)	Base (m bgl)	Generalised Geology	Locations
0.00	0.30-0.41	TOPSOIL: Brown slightly silty clay with rootlets. Gravelly silty clay encountered within WS18 only.	ALL
0.30-0.41	1.40-4.45	UPPER TUNBRIDGE WELLS SAND FORMATION: Soft to very stiff sandy CLAY and slightly gravelly sandy CLAY. Gravels are of fine to coarse, sub-angular of flint, sandstone and mudstone.	ALL
0.95-3.05	1.40-3.10	UPPER TUNBRIDGE WELLS SAND FORMATION: Orange brown gravelly SAND, very clayey SAND and SAND.	WS12, WS18

Table 4.1 Summary of Ground Conditions

Figure 2. GI Report extract (Ground Conditions Summary)

3.2 Groundwater and Infiltration Testing

- 3.2.1 Ground Investigation Report states that:
'Groundwater was encountered within WS15 and WS16 during the investigation at depths of between 1.10m and 2.10m bgl, then standing at depths of between 2.0m and 2.1m BGL after 20 minutes. Standpipes were installed within all boreholes to depths of between 1.4m and 4.45m BGL with twelve monitoring visits planned over the winter period. To date the first two visits have been undertaken. Groundwater was recorded at depths of between 0.0mBGL and 2.0mBGL.'
- 3.2.2 Soakaway testing did not form part of the remit. GI report states that: (...) *however, based on the encountered predominantly cohesive ground conditions, conventional shallow soakaways are unlikely to be feasible and an alternative means of storm water disposal should be sought'*
- 3.2.3 It should also be noted that, due to the elevated groundwater levels, the use of soakaways has been ruled out as a viable drainage solution.
- 3.2.4 An additional Ground Gas Assessment and Winter Groundwater Monitoring Statement was provided in May 2024 following completion of the groundwater monitoring period. The assessment identified groundwater levels ranging between 1.4m and 4.0m below ground level (BGL). Groundwater was also recorded at varying depths between 0mBGL and 2.25mBGL.
- 3.2.5 Copies of the ground investigation and winter groundwater monitoring reports are provided in Appendix D at the rear of this statement.

4 Proposed Development

4.1 Description

- 4.1.1 The development proposal is for the erection of up to 200 dwellings, a community building, new vehicular access point from the A272 Cowfold Road and pedestrian and cycle only access to The Street.
- 4.1.2 The community building will be located within the Northern Parcel.

4.2 Impermeable Areas

- 4.2.1 The total impermeable area of the various elements of the development is summarised as follows. An allowance of 10% Urban Creep has been included within the impermeable areas stated:

4.2.1.1 Northern Parcel

Item	Area (m ²)	Area (ha)
Northern Parcel Area (Greenfield runoff)	70,315	7.03
Roof Areas	7,340	0.73
Roof Areas with 10% Urban Creep	8,075	0.807
Hardstanding	17,441	1.744
Attenuation Basins	4,632	0.462
Total	30,148	3.015

4.2.1.2 Southern Parcel

Item	Area (m ²)	Area (ha)
Southern Parcel Area (Greenfield runoff)	84,445	8.44
Roof Areas	6,670	0.667
Roof Areas with 10% Urban Creep	7,337	0.7337
Hardstanding	14,960	1.496
Attenuation Basins	6,100	0.61
Total	28,397	2.84

- 4.2.2 The landscaped areas from which surface water runoff is to be collected by cut-off swales or filter drains have been included within the on-site drainage calculations. A runoff coefficient (Cv) of **0.4** has been adopted for these landscaped areas to represent the greenfield runoff characteristics of the contributing catchments.

4.3 Rainfall Climate Change Allowance

- 4.3.1 The latest climate change allowances (available online <https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances>), which supports the NPPF in making an allowance for climate change within drainage design. The allowances provide predictions of anticipated change for peak river flow by river basin district and peak rainfall intensity.
- 4.3.2 The table below shows the anticipated changes in extreme rainfall intensity. The anticipated increases in peak rainfall across the Adur and Ouse Management Catchment for the 1% annual exceedance rainfall event are as follows:

Epoch	Central Allowance	Upper End Allowance
2050s	20%	45%
2070s	25%	45%

Figure 3. Climate Change Allowances

**Use '2050s' for development with a lifetime up to 2060 and use the 2070s epoch for development with a lifetime between 2061 and 2125.*

- 4.3.3 Based on the latest allowances and a life span of the development of 70 to 100 years, an increase of 45% has been applied to peak rainfall to this development.

5 Surface Water Drainage

- 5.1.1 CIRIA report C753 The SuDS Manual provides guidance on surface water drainage. The aim for surface water runoff is to match greenfield runoff rates and volumes, where reasonably achievable.
- 5.1.2 For surface water discharge, the drainage hierarchy notes the following list of drainage options in order of preference:
- Infiltration to ground
 - Discharge to a watercourse
 - Discharge to a surface water sewer
 - Discharge to a foul water sewer
- 5.1.3 Based on the findings of the ground investigation, which confirmed a low potential for infiltration and the presence of high groundwater levels, the use of infiltration as a surface water disposal method has been ruled out. The proposed surface water drainage strategy therefore involves discharging surface water runoff to the existing watercourses.
- 5.1.4 The proposed SuDS system has been designed to provide sufficient attenuation storage to accommodate the 1 in 100-year storm event, including an additional 45% allowance for the predicted effects of future climate change.
- 5.1.5 The proposed on-site attenuation features have also been sized to accommodate surface water runoff collected from the landscaped areas via cut-off swales or filter drains. The Environment Agency (EA) states that the *Risk of Flooding from Surface Water* map identifies areas where surface water flooding may occur when rainfall does not drain away through positive drainage systems or infiltrate into the ground, but instead ponds on or flows over the ground surface. The proposed drainage strategy has therefore been designed to intercept, convey and attenuate surface water runoff from both the developed and landscaped areas, ensuring there is no unacceptable risk of surface water flooding to the

proposed dwellings while also reducing the risk of surface water flooding across the wider site.

- 5.1.6 Any surface water runoff generated on the site will be collected, conveyed and attenuated within the proposed drainage system before being discharged at a restricted rate to the receiving watercourse via the on-site surface water drainage network.
- 5.1.7 The proposed site levels have been designed, where practicable, to facilitate the implementation of SuDS features and to ensure that surface water runoff is effectively conveyed from across the site to the main on-site surface water drainage network.
- 5.1.8 It is proposed that surface water runoff from the northern parcel will be collected and discharged to the existing culvert beneath Foxhole Lane, located to the west of the proposed development, which subsequently outfalls to the existing watercourse. The discharge rate will be restricted to the existing greenfield QBAR runoff rate, with a maximum discharge of 38.7 l/s.
- 5.1.9 Surface water runoff from the southern parcel will be collected and discharged to the existing ditch located adjacent to Cowfold Road along the southern boundary of the site. The ditch subsequently discharges to the existing sewer network in Cowfold Road. The discharge rate will be restricted to the existing greenfield QBAR runoff rate, with a maximum discharge of 46.4 l/s.
- 5.1.10 Surface water runoff shall be restricted throughout the proposed drainage network. The manholes where restricted discharge is required are listed in the table below.

Manhole Number	Flow Restriction	Other Manhole Details
Northern Parcel		
S72 (Hydro-brake)	38.7 l/s	Design depth: 1.5 CL 32.000mAOD IL 30.510mAOD
S83 (Orifice)	Orifice Diameter 100mm	CL 35.345mAOD IL 34.180mAOD
Southern Parcel		
S14 (Orifice)	Orifice Diameter 100mm	CL 37.00mAOD IL 35.99mAOD
S31 (Orifice)	Orifice Diameter 75mm	CL 35.200mAOD IL 34.00mAOD
S11 (Orifice)	Orifice Diameter 50mm	CL 34.300mAOD IL 33.300mAOD
S20 (Hydro-brake)	25.0 l/s	Design depth: 1.6 CL 26.348mAOD IL 23.525mAOD
S2 (Hydro-brake)	46.4 l/s	Design depth: 1.5 CL 23.000mAOD IL 21.832mAOD

- 5.1.11 All proposed attenuation basins have been designed as dry basins that will normally remain empty, providing usable landscaped areas during ordinary weather conditions while temporarily storing surface water runoff during rainfall events. The basins provide sufficient attenuation storage for all design rainfall events, including the critical 1 in 100-year storm event with an appropriate allowance for climate change. The basins have been designed with maximum side slopes of 1 in 3 to ensure they can be safely accessed, maintained and integrated into the overall landscape design.

6 Foul Drainage

- 6.1.1 There is currently no foul drainage network within the site. During the planning application stage in February 2024, Ramboll undertook discussions with Southern Water regarding foul drainage proposals, which initially comprised two separate discharge connections to serve the two development parcels. Further discussions took place in September 2024, during which a revised proposal for a single foul drainage connection serving the entire development, discharging to the existing sewer on Cowfold Road, was presented. Southern Water subsequently confirmed that there is adequate capacity within the local sewerage network to accommodate the proposed foul discharge rate of 1.8 l/s at this location.
- 6.1.2 The foul drainage network has been designed in accordance with the Design and Construction Guidance (DCG). In accordance with the DCG, a foul flow of 4,000 litres per dwelling per day has been adopted. Based on a development of 200 dwellings, this equates to an estimated peak foul flow of 9.3 l/s.
- 6.1.3 A more detailed assessment was subsequently undertaken in accordance with the Southern Water Foul Sewerage Modelling Criteria, which identified a peak discharge rate of 1.8 l/s, with no allowance made for misconnected surface water. This discharge rate has been agreed with Southern Water and forms the basis of the proposed foul drainage strategy.
- 6.1.4 It is proposed that foul water from the southern parcel will be collected and discharged by gravity to the existing foul sewer in Cowfold Road, located to the south of the site. Due to the site topography, foul water from the northern parcel will be collected and conveyed to a proposed on-site foul pumping station located in the north-western corner of the site. From the pumping station, the wastewater will be pumped via a rising main and discharged into the gravity foul sewer serving the southern parcel within the main access road. The combined foul flows will then discharge by gravity to the existing Southern Water sewer in Cowfold Road.
- 6.1.5 It should be noted that the proposed separate surface water and foul drainage networks will capture and divert both surface water and foul flows away from The Street. This will significantly reduce the volume and peak rate of surface water entering the existing combined sewer network in The Street, thereby reducing the risk of sewer flooding and increasing the available capacity within the local sewerage network.
- 6.1.6 A copy of the proposed impermeable area plan, drainage plan, construction details and drainage calculations can be found in Appendix E at the rear of this report.

7 Water Quality

- 7.1.1 The proposed development is residential. In accordance with CIRIA SuDS Manual 2015 (Report C753), the pollution hazard level for this type of development is very low to medium.
- 7.1.2 The surface water scheme has been designed to include mitigation to ensure that surface water is suitably treated, and any pollution risk adequately managed prior to discharge.
- 7.1.3 Surface water runoff from the main access road, classified as medium risk, is proposed to be routed through the proposed basins before being discharged into the existing watercourse. Runoff will be treated through the dry basins/swales preventing the discharge of total suspended solids (TSS), metals, and hydrocarbons into the existing watercourse downstream.
- 7.1.4 Table 26.2 in Chapter 26 of CIRIA report C753 The SuDS Manual provides Pollution Hazard Indices for varying land types. Those of relevance to the development proposals are as follows:

	Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Residential roofs	Very Low	0.2	0.2	0.05
Individual property driveways, residential car park, low-traffic roads (below 300 movements)	Low	0.5	0.4	0.4
Commercial yard and delivery areas, non-residential car parking with frequent change, all roads except low traffic roads and trunk roads/motorways	Medium	0.7	0.6	0.7

Figure 4. Pollution Hazard Indices

- 7.1.5 Pollution Mitigation Indices for each SuDS component within the proposal are as follows:

Type of Mitigation	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Reno mattress (Basin Inlet - filter drain)	0.4	0.4	0.4
Detention Basin (1)	0.5 x 0.5	0.5 x 0.5	0.5 x 0.6
Detention Basin (2)	0.5 x 0.5	0.5 x 0.5	0.5 x 0.6
Total:	0.9	0.9	1.0

Figure 5. Pollution Mitigation Indices

- 7.1.6 A factor of 0.5 has been used to account for the reduced performance of secondary component associated with already reduced inflow concentrations.
- 7.1.7 The surface water drainage design combines the use of reno mattress and series of attenuation basins. The proposed drainage meets the target treatment level required for a site with Pollution Hazard Level of 'Very Low to Medium Risk'.

8 Conclusions

- 8.1.1 The proposed development comprises the erection of 200 residential dwellings, together with a community building incorporating land for education provision, associated access roads, parking, landscaping and other ancillary infrastructure.
- 8.1.2 A sustainable drainage system (SuDS) is proposed in accordance with the requirements of national and local planning policy, providing a sustainable approach to the management of surface water runoff.
- 8.1.3 The development is divided into two parcels, with surface water managed independently within each parcel before discharging to separate receiving watercourses.
- 8.1.4 Surface water runoff from the Southern Parcel will discharge to an existing ditch located adjacent to the southern boundary of the site, which subsequently outfalls to the existing sewer network in Cowfold Road. Surface water runoff from the Northern Parcel will discharge via the existing culvert beneath Foxhole Lane, located to the west of the site, which subsequently outfalls to the existing unnamed watercourse.
- 8.1.5 The proposed surface water drainage strategy has been designed to restrict runoff to the site's existing greenfield runoff rate of 5.5 l/s/ha. Accordingly, the proposed discharge rates have been restricted to 38.7 l/s for the Northern Parcel and 46.4 l/s for the Southern Parcel.
- 8.1.6 The proposed surface water drainage system has been designed to collect runoff from the developed areas as well as the surrounding landscaped and open space areas via cut-off swales and filter drains, conveying flows through the on-site drainage network for attenuation and controlled discharge. As a result, the proposed strategy will improve the existing surface water management regime and reduce the risk of surface water flooding identified within the Flood Risk Assessment and Environment Agency surface water flood mapping.
- 8.1.7 Foul water from the development will discharge to the existing Southern Water foul sewer located within Cowfold Road, to the south of the site. Due to the site's topography, foul water from the Northern Parcel will be collected and conveyed to a proposed on-site foul pumping station, from where it will be pumped via a rising main into the gravity foul sewer serving the Southern Parcel before discharging to the existing Southern Water network.
- 8.1.8 A detailed assessment undertaken in accordance with the Southern Water Foul Sewerage Modelling Criteria identified a peak foul discharge rate of 1.8 l/s, with no allowance made for misconnected surface water. This discharge rate was agreed with Southern Water during the outline planning application stage and forms the basis of the proposed foul drainage strategy.
- 8.1.9 Water quality improvement will be provided as part of the SuDS system proposed.

Appendices

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