1593_251010          1OTH OCTOBER 2025

**Mr Paul Burgess**
Lewis & Co
2 Port Hall Road, Brighton, BN1 5PD

**BY EMAIL ONLY**

Dear Paul

**1593 - 203 JUNCTION ROAD BURGESS HILL RH15 0NX**

Please see attached our Infiltration Testing Summary for your issue to the planners in response to their email dated 2nd October 2025.

I do note there is a typo on the drawings which we indicate the rate as $1.35 \times 10\text{-}5$ m/s whereas calculation indicated this as 1.27 but the error makes little difference in the storm water strategy. I have altered the main drawing to suit this which is also attached and perhaps you can relay this to the planning officer.

If you have any queries, please contact me on 0754 658 3150.

Kind regards,

**Simon Dent**
**For Simon Dent Associates**

# INFILTRATION TESTING SUMMARY

## FOR 201 JUNCTION ROAD BURGESS HILL WEST SUSSEX RH15 0NX

### Preliminary Work

BRE 365 soakaway testing was requested by SDA to be carried out for the site. The client carried these out with SDA present partly. The test results were provided for two differing locations as shown on the test pit plan below.



The British Geological Society (BGS) website indicated soils as Wealdstone clay which gave surprisingly good results. The clay was silty and with subsequent tests still proved viable for soakage design even at depth.

Each pit was excavated to 2.0m x 1.0m x 1.5m with the sides holding firm throughout.

Testing was carried out in November 2019 within two pits within.

No groundwater was encountered during the tests which were carried out in November 2019.

Each pit was filled and emptied three times (albeit starting depths reduced due to time taken on the initial test) over the consecutive days (due to the time taken to empty) with the average result used with a factor of safety to determined new soakaway and drainage blanket sizing.

Initially depths were taken at 10 min increments but subsequent recording were taken on the hour due to time taken. A one-day limit was placed on each test.

Where the test does not fully drain, tests results are established using the water level drop from 75% to 25% of the actual drained depth as is permitted in the BRE 365 test. Test locations are in the location at which the design required at the time.

**Fig 1 - Soakaway test pits in 2019**

## Results

The client was provided with the forms to fill in the SDA test record depths for the given time and infiltration results established therefrom. The test results are shown attached with the results summary below with the resultant chosen design infiltration rate.

| Pit and Test No. | Test result |
|---|---|
| Pit 1 Test 1 | $1.60 \times 10^{-5}$ m/s |
| Pit 1 Test 2 | $0.89 \times 10^{-5}$ m/s |
| Pit 1 Test 3 | $0.96 \times 10^{-5}$ m/s |
| Pit 2 Test 1 | $1.48 \times 10^{-5}$ m/s |
| Pit 2 Test 2 | $0.89 \times 10^{-5}$ m/s |
| Pit 2 Test 3 | $1.83 \times 10^{-5}$ m/s |

With the resultant test rates, the average of the six was taken resulting in the design figure of $1.27 \times 10^{-5}$ m/s.
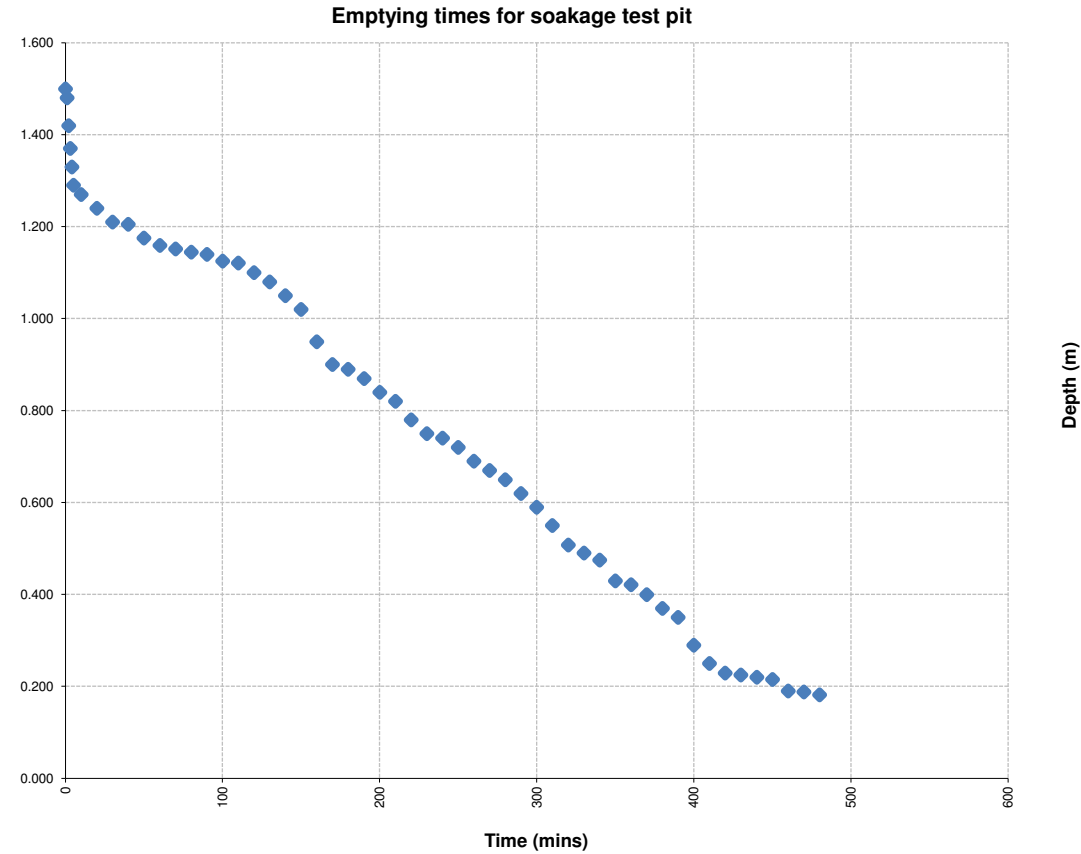
This rate has been factored by 1.5 for design purposes.

**SIMON DENT ASSOCIATES**

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| Project Title: | 203 Junction Road Burgess Hill - PIT No. 1 - Test 1 | No.: | 1593 |
|---|---|---|---|

## Infiltration test and rate calculation

| 1. Permeability rate calculations | | 2. Test Outflow results | | 3. Infiltration coefficient (q) (m/h) | |
|---|---|---|---|---|---|
| Test pit L: | 2.00 | Tot vol.: | 3 | $a_{s50}$ (sq.m): | 4.50 | Rate (q) provided by SI= | |
| Test pit W: | 1.00 | $a_{p50}$: | 6.5 | Inflow (cu.m): | #REF! | Calculated Soil Rate (f)= | 0.05793 |
| Test pit D: | 1.50 | $V_{p75-25}$: | 1.318 | Outflow(cu.m): | 1.50 | | |
| Free vol (%): | 100 | $t_{p75-25}$: | 210 | Storage(cu.m): | #REF! | | |
| Effective depth: | | 1.318 | | | | | |
| Infiltration rate (f): | | 1.609E-05 | | | | | |

**Log of falling head test**

| Time | Depth | Time | Depth | Time | Depth | Time | Depth | Time | Depth |
|---|---|---|---|---|---|---|---|---|---|
| 5 | 1290 | 105 | | 205 | | 305 | | 405 | |
| 10 | 1270 | 110 | 1121 | 210 | 820 | 310 | 550 | 410 | 250 |
| 15 | | 115 | | 215 | | 315 | | 415 | |
| 20 | 1240 | 120 | 1100 | 220 | 780 | 320 | 508 | 420 | 229 |
| 25 | | 125 | | 225 | | 325 | | 425 | |
| 30 | 1210 | 130 | 1080 | 230 | 750 | 330 | 490 | 430 | 225 |
| 35 | | 135 | | 235 | | 335 | | 435 | |
| 40 | 1205 | 140 | 1050 | 240 | 740 | 340 | 475 | 440 | 220 |
| 45 | | 145 | | 245 | | 345 | | 445 | |
| 50 | 1175 | 150 | 1020 | 250 | 720 | 350 | 430 | 450 | 215 |
| 55 | | 155 | | 255 | | 355 | | 455 | |
| 60 | 1159 | 160 | 950 | 260 | 690 | 360 | 421 | 460 | 190 |
| 65 | | 165 | | 265 | | 365 | | 465 | |
| 70 | 1152 | 170 | 900 | 270 | 670 | 370 | 400 | 470 | 188 |
| 75 | | 175 | | 275 | | 375 | | 475 | |
| 80 | 1145 | 180 | 890 | 280 | 650 | 380 | 370 | 480 | 182 |
| 85 | | 185 | | 285 | | 385 | | 485 | |
| 90 | 1140 | 190 | 870 | 290 | 620 | 390 | 350 | 490 | |
| 95 | | 195 | | 295 | | 395 | | 495 | |
| 100 | 1125 | 200 | 840 | 300 | 590 | 400 | 290 | 500 | |



**Emptying times for soakage test pit**

Depth (m) vs Time (mins)

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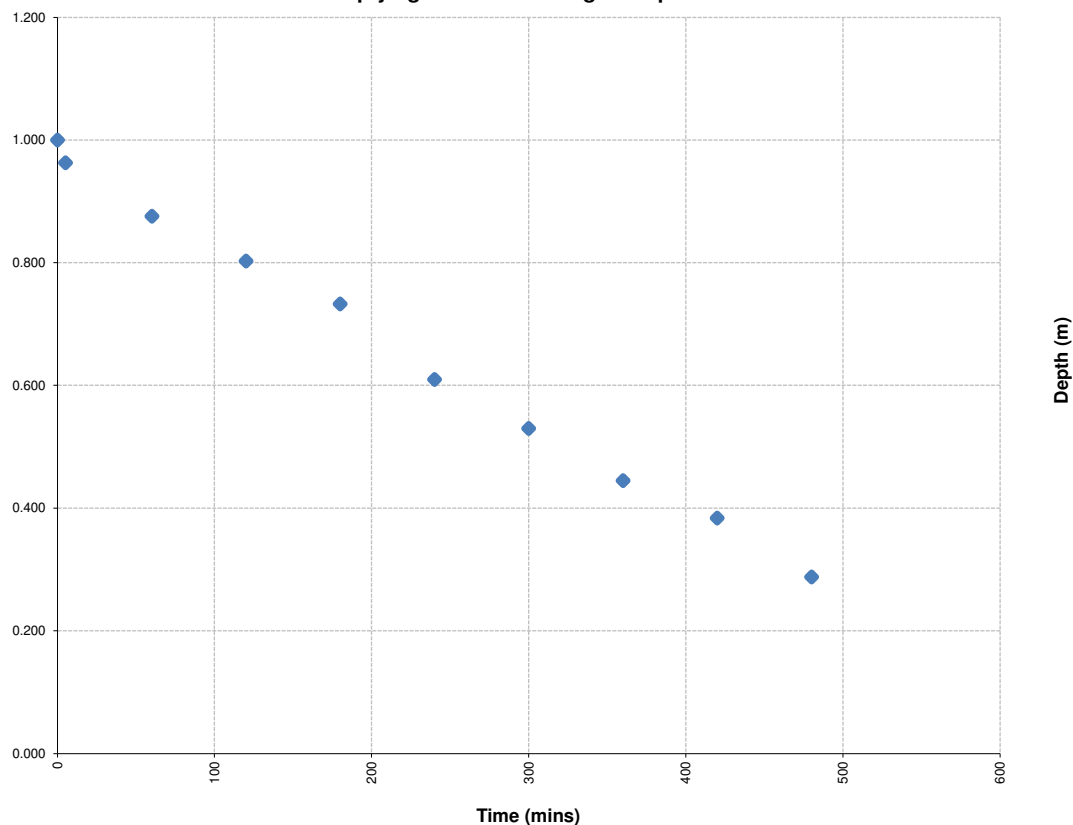| Project Title: | 203 Junction Road Burgess Hill - PIT No. 1 - Test 2 | No.: | 1593 |
|---|---|---|---|

## Infiltration test and rate calculation

| 1. Permeability rate calculations | | | | 2. Test Outflow results | | 3. Infiltration coefficient (q) (m/h) | |
|---|---|---|---|---|---|---|---|
| Test pit L: | 2.00 | Tot vol.: | 3 | $a_{s50}$ (sq.m): | 4.50 | Rate (q) provided by SI= | |
| Test pit W: | 1.00 | $a_{p50}$ | 6.5 | Inflow (cu.m): | #REF! | Calculated Soil Rate (f)= | 0.03206 |
| Test pit D: | 1.50 | $V_{p75-25}$ | 0.712 | Outflow(cu.m): | 1.50 | | |
| Free vol (%): | 100 | $t_{p75-25}$ | 205 | Storage(cu.m): | #REF! | | |
| Effective depth: | | 0.712 | | | | | |
| Infiltration rate (f): | | 8.906E-06 | | | | | |

### Log of falling head test

| Time | Depth | Time | Depth | Time | Depth | Time | Depth | Time | Depth |
|---|---|---|---|---|---|---|---|---|---|
| 5 | 963 | 105 | | 205 | | 305 | | 405 | |
| 10 | | 110 | | 210 | | 310 | | 410 | |
| 15 | | 115 | | 215 | | 315 | | 415 | |
| 20 | | 120 | 803 | 220 | | 320 | | 420 | 384 |
| 25 | | 125 | | 225 | | 325 | | 425 | |
| 30 | | 130 | | 230 | | 330 | | 430 | |
| 35 | | 135 | | 235 | | 335 | | 435 | |
| 40 | | 140 | | 240 | 610 | 340 | | 440 | |
| 45 | | 145 | | 245 | | 345 | | 445 | |
| 50 | | 150 | | 250 | | 350 | | 450 | |
| 55 | | 155 | | 255 | | 355 | | 455 | |
| 60 | 876 | 160 | | 260 | | 360 | 445 | 460 | |
| 65 | | 165 | | 265 | | 365 | | 465 | |
| 70 | | 170 | | 270 | | 370 | | 470 | |
| 75 | | 175 | | 275 | | 375 | | 475 | |
| 80 | | 180 | 733 | 280 | | 380 | | 480 | 288 |
| 85 | | 185 | | 285 | | 385 | | 485 | |
| 90 | | 190 | | 290 | | 390 | | 490 | |
| 95 | | 195 | | 295 | | 395 | | 495 | |
| 100 | | 200 | | 300 | 530 | 400 | | 500 | |

### Emptying times for soakage test pit

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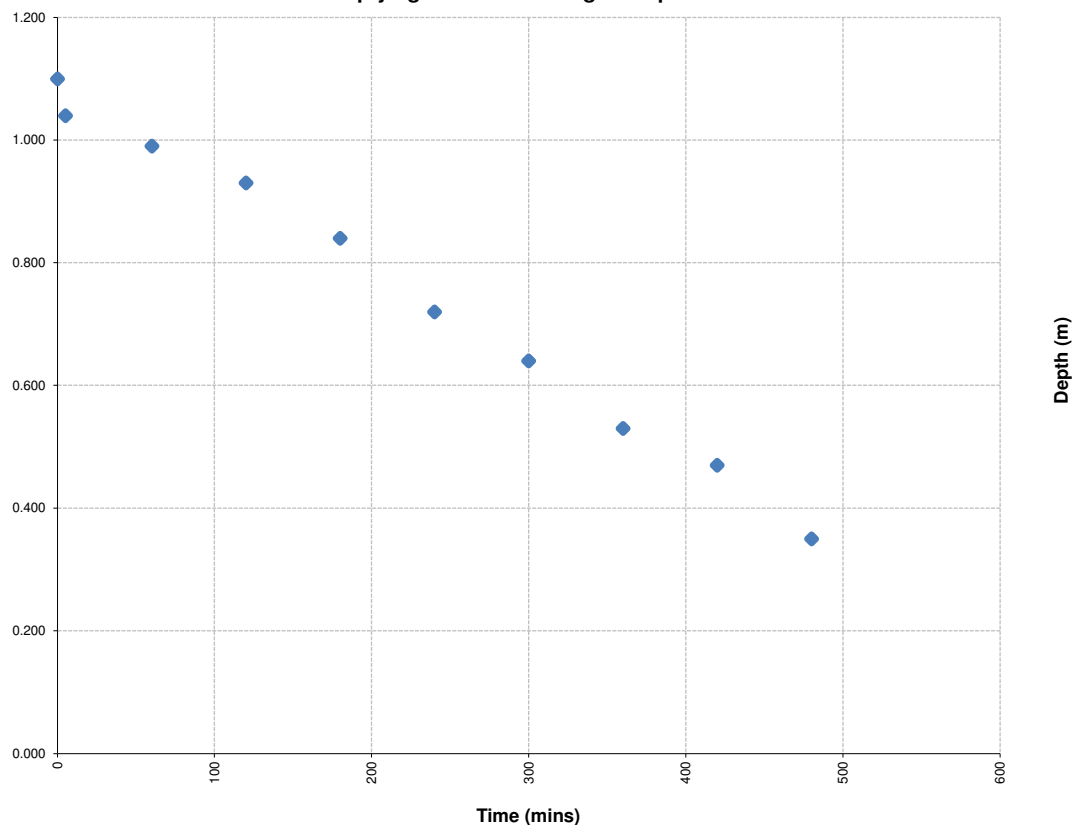| Project Title: | 203 Junction Road Burgess Hill - PIT No. 1 - Test 3 | No.: | 1593 |
|---|---|---|---|

## Infiltration test and rate calculation

| 1. Permeability rate calculations | | | | 2. Test Outflow results | | 3. Infiltration coefficient (q) (m/h) | |
|---|---|---|---|---|---|---|---|
| Test pit L: | 2.00 | Tot vol.: | 3 | $a_{s50}$ (sq.m): | 4.50 | Rate (q) provided by SI= | |
| Test pit W: | 1.00 | $a_{p50}$: | 6.5 | Inflow (cu.m): | #REF! | Calculated Soil Rate (f)= | 0.03462 |
| Test pit D: | 1.50 | $V_{p75-25}$: | 0.75 | Outflow(cu.m): | 1.50 | | |
| Free vol (%): | 100 | $t_{p75-25}$: | 200 | Storage(cu.m): | #REF! | | |
| Effective depth: | | 0.750 | | | | | |
| Infiltration rate (f): | | 9.615E-06 | | | | | |

**Log of falling head test**

| Time | Depth | Time | Depth | Time | Depth | Time | Depth | Time | Depth |
|---|---|---|---|---|---|---|---|---|---|
| 5 | 1040 | 105 | | 205 | | 305 | | 405 | |
| 10 | | 110 | | 210 | | 310 | | 410 | |
| 15 | | 115 | | 215 | | 315 | | 415 | |
| 20 | | 120 | 930 | 220 | | 320 | | 420 | 470 |
| 25 | | 125 | | 225 | | 325 | | 425 | |
| 30 | | 130 | | 230 | | 330 | | 430 | |
| 35 | | 135 | | 235 | | 335 | | 435 | |
| 40 | | 140 | | 240 | 720 | 340 | | 440 | |
| 45 | | 145 | | 245 | | 345 | | 445 | |
| 50 | | 150 | | 250 | | 350 | | 450 | |
| 55 | | 155 | | 255 | | 355 | | 455 | |
| 60 | 990 | 160 | | 260 | | 360 | 530 | 460 | |
| 65 | | 165 | | 265 | | 365 | | 465 | |
| 70 | | 170 | | 270 | | 370 | | 470 | |
| 75 | | 175 | | 275 | | 375 | | 475 | |
| 80 | | 180 | 840 | 280 | | 380 | | 480 | 350 |
| 85 | | 185 | | 285 | | 385 | | 485 | |
| 90 | | 190 | | 290 | | 390 | | 490 | |
| 95 | | 195 | | 295 | | 395 | | 495 | |
| 100 | | 200 | | 300 | 640 | 400 | | 500 | |

**Emptying times for soakage test pit**



Depth (m) vs Time (mins)

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|---|---|---|---|

## Infiltration test and rate calculation

| 1. Permeability rate calculations | | 2. Test Outflow results | | 3. Infiltration coefficient (q) (m/h) | |
|---|---|---|---|---|---|
| Test pit L: | 2.00 | Tot vol.: | 3 | $a_{s50}$ (sq.m): | 4.50 | Rate (q) provided by SI= | |
| Test pit W: | 1.00 | $a_{p50}$: | 6.5 | Inflow (cu.m): | #REF! | Calculated Soil Rate (f)= | 0.05354 |
| Test pit D: | 1.50 | $V_{p75-25}$: | 1.218 | Outflow(cu.m): | 1.50 | | |
| Free vol (%): | 100 | $t_{p75-25}$: | 210 | Storage(cu.m): | #REF! | | |
| Effective depth: | | 1.218 | | | | | |
| Infiltration rate (f): | | 1.487E-05 | | | | | |

**Log of falling head test**

| Time | Depth | Time | Depth | Time | Depth | Time | Depth | Time | Depth |
|---|---|---|---|---|---|---|---|---|---|
| 5 | 1300 | 105 | | 205 | | 305 | | 405 | |
| 10 | 1290 | 110 | 1164 | 210 | 939 | 310 | 600 | 410 | 423 |
| 15 | | 115 | | 215 | | 315 | | 415 | |
| 20 | 1250 | 120 | 1159 | 220 | 880 | 320 | 596 | 420 | 395 |
| 25 | | 125 | | 225 | | 325 | | 425 | |
| 30 | 1235 | 130 | 1148 | 230 | 860 | 330 | 550 | 430 | 390 |
| 35 | | 135 | | 235 | | 335 | | 435 | |
| 40 | 1210 | 140 | 1141 | 240 | 845 | 340 | 535 | 440 | 330 |
| 45 | | 145 | | 245 | | 345 | | 445 | |
| 50 | 1196 | 150 | 1120 | 250 | 810 | 350 | 510 | 450 | 300 |
| 55 | | 155 | | 255 | | 355 | | 455 | |
| 60 | 1179 | 160 | 1050 | 260 | 789 | 360 | 500 | 460 | 298 |
| 65 | | 165 | | 265 | | 365 | | 465 | |
| 70 | 1176 | 170 | 1000 | 270 | 750 | 370 | 496 | 470 | 279 |
| 75 | | 175 | | 275 | | 375 | | 475 | |
| 80 | 1174 | 180 | 960 | 280 | 690 | 380 | 476 | 480 | 250 |
| 85 | | 185 | | 285 | | 385 | | 485 | |
| 90 | 1171 | 190 | 958 | 290 | 676 | 390 | 430 | 490 | |
| 95 | | 195 | | 295 | | 395 | | 495 | |
| 100 | 1167 | 200 | 949 | 300 | 620 | 400 | 429 | 500 | |



**Emptying times for soakage test pit**
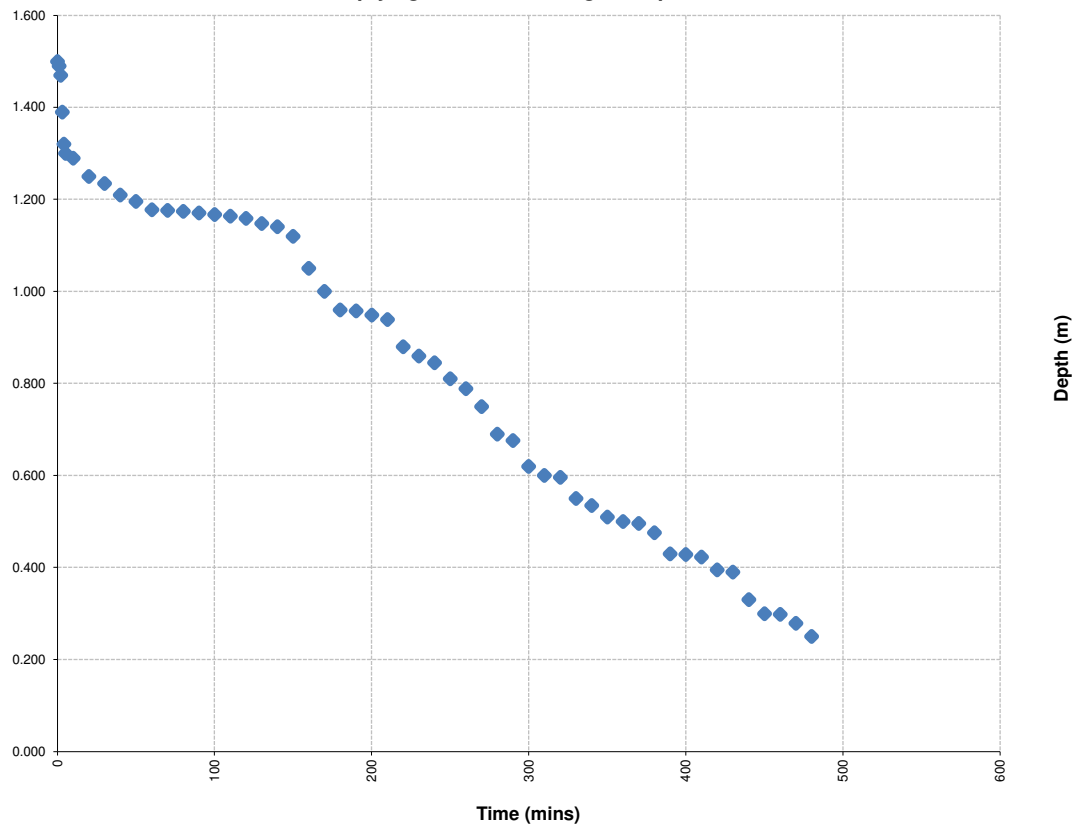
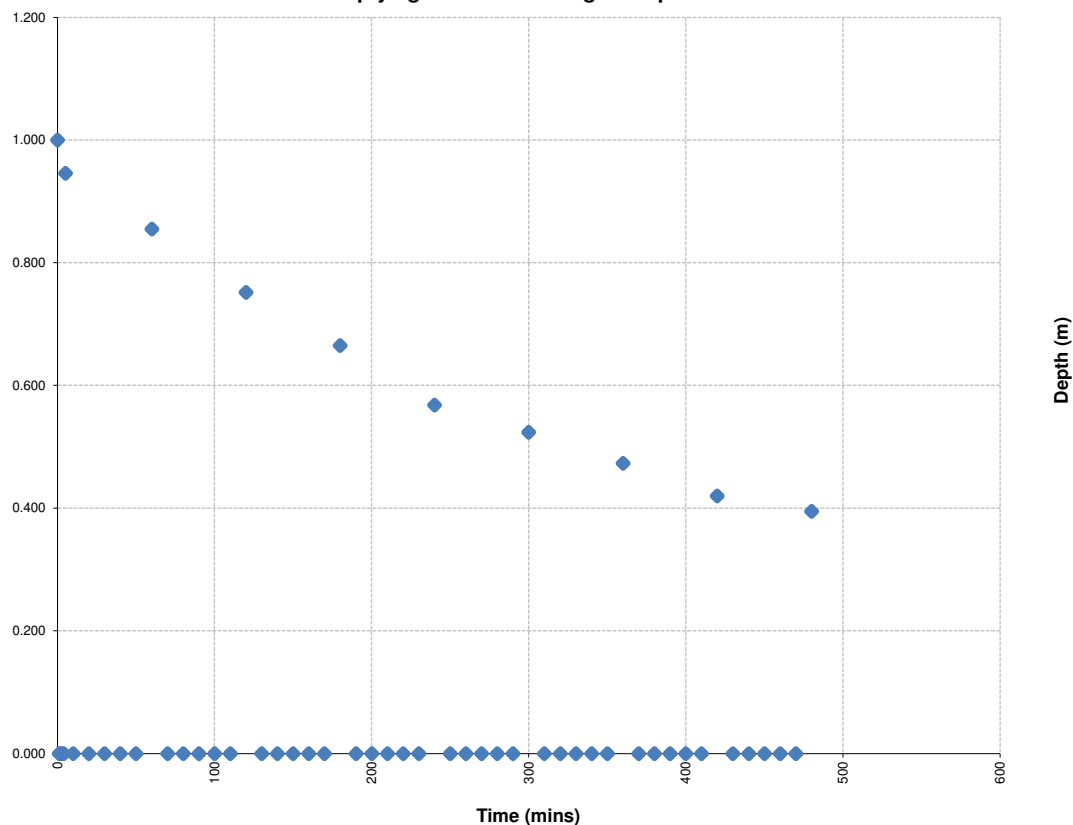| Project Title: | 203 Junction Road Burgess Hill - PIT No. 2 - Test 2 | No.: | 1593 |
|---|---|---|---|

## Infiltration test and rate calculation

| 1. Permeability rate calculations | | | | 2. Test Outflow results | | 3. Infiltration coefficient (q) (m/h) | |
|---|---|---|---|---|---|---|---|
| Test pit L: | 2.00 | Tot vol.: | 3 | $a_{s50}$ (sq.m): | 4.50 | Rate (q) provided by SI= | |
| Test pit W: | 1.00 | $a_{p50}$ | 6.5 | Inflow (cu.m): | #REF! | Calculated Soil Rate (f)= | 0.03206 |
| Test pit D: | 1.50 | $V_{p75-25}$ | 0.712 | Outflow(cu.m): | 1.50 | | |
| Free vol (%): | 100 | $t_{p75-25}$ | 205 | Storage(cu.m): | #REF! | | |
| Effective depth: | | | 0.712 | | | | |
| Infiltration rate (f): | | 8.906E-06 | | | | | |

**Log of falling head test**

| Time | Depth | Time | Depth | Time | Depth | Time | Depth | Time | Depth |
|---|---|---|---|---|---|---|---|---|---|
| 5 | 963 | 105 | | 205 | | 305 | | 405 | |
| 10 | | 110 | | 210 | | 310 | | 410 | |
| 15 | | 115 | | 215 | | 315 | | 415 | |
| 20 | | 120 | 803 | 220 | | 320 | | 420 | 384 |
| 25 | | 125 | | 225 | | 325 | | 425 | |
| 30 | | 130 | | 230 | | 330 | | 430 | |
| 35 | | 135 | | 235 | | 335 | | 435 | |
| 40 | | 140 | | 240 | 610 | 340 | | 440 | |
| 45 | | 145 | | 245 | | 345 | | 445 | |
| 50 | | 150 | | 250 | | 350 | | 450 | |
| 55 | | 155 | | 255 | | 355 | | 455 | |
| 60 | 876 | 160 | | 260 | | 360 | 445 | 460 | |
| 65 | | 165 | | 265 | | 365 | | 465 | |
| 70 | | 170 | | 270 | | 370 | | 470 | |
| 75 | | 175 | | 275 | | 375 | | 475 | |
| 80 | | 180 | 733 | 280 | | 380 | | 480 | 288 |
| 85 | | 185 | | 285 | | 385 | | 485 | |
| 90 | | 190 | | 290 | | 390 | | 490 | |
| 95 | | 195 | | 295 | | 395 | | 495 | |
| 100 | | 200 | | 300 | 530 | 400 | | 500 | |

### Emptying times for soakage test pit



Depth (m) vs Time (mins)

**SIMON DENT ASSOCIATES**

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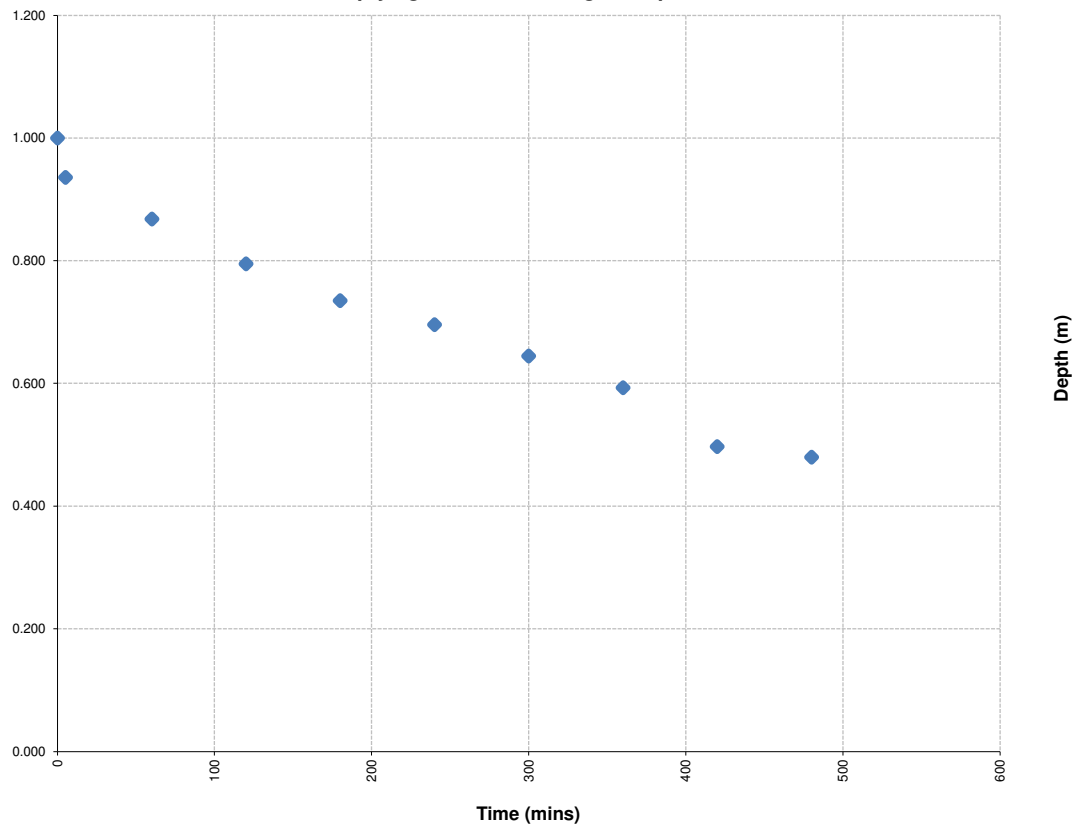| Project Title: | 203 Junction Road Burgess Hill - PIT No. 2 - Test 3 | No.: | 1593 |
|---|---|---|---|

## Infiltration test and rate calculation

| 1. Permeability rate calculations | | | | 2. Test Outflow results | | 3. Infiltration coefficient (q) (m/h) | |
|---|---|---|---|---|---|---|---|
| Test pit L: | 2.00 | Tot vol.: | 3 | $a_{s50}$ (sq.m): | 4.50 | Rate (q) provided by SI= | |
| Test pit W: | 1.00 | $a_{p50}$: | 6.5 | Inflow (cu.m): | #REF! | Calculated Soil Rate (f)= | 0.06593 |
| Test pit D: | 1.50 | $V_{p75-25}$: | 1.5 | Outflow(cu.m): | 1.50 | | |
| Free vol (%): | 100 | $t_{p75-25}$: | 210 | Storage(cu.m): | #REF! | | |
| Effective depth: | | 1.500 | | | | | |
| Infiltration rate (f): | | 1.832E-05 | | | | | |

**Log of falling head test**

| Time | Depth | Time | Depth | Time | Depth | Time | Depth | Time | Depth |
|---|---|---|---|---|---|---|---|---|---|
| 5 | 1040 | 105 | | 205 | | 305 | | 405 | |
| 10 | | 110 | | 210 | | 310 | | 410 | |
| 15 | | 115 | | 215 | | 315 | | 415 | |
| 20 | | 120 | 930 | 220 | | 320 | | 420 | 470 |
| 25 | | 125 | | 225 | | 325 | | 425 | |
| 30 | | 130 | | 230 | | 330 | | 430 | |
| 35 | | 135 | | 235 | | 335 | | 435 | |
| 40 | | 140 | | 240 | 720 | 340 | | 440 | |
| 45 | | 145 | | 245 | | 345 | | 445 | |
| 50 | | 150 | | 250 | | 350 | | 450 | |
| 55 | | 155 | | 255 | | 355 | | 455 | |
| 60 | 990 | 160 | | 260 | | 360 | 530 | 460 | |
| 65 | | 165 | | 265 | | 365 | | 465 | |
| 70 | | 170 | | 270 | | 370 | | 470 | |
| 75 | | 175 | | 275 | | 375 | | 475 | |
| 80 | | 180 | 840 | 280 | | 380 | | 480 | 350 |
| 85 | | 185 | | 285 | | 385 | | 485 | |
| 90 | | 190 | | 290 | | 390 | | 490 | |
| 95 | | 195 | | 295 | | 395 | | 495 | |
| 100 | | 200 | | 300 | 640 | 400 | | 500 | |



**Emptying times for soakage test pit**

Depth (m) vs Time (mins)

# FOUL & STORM WATER DESIGN - DRAINAGE STRATEGY

**GENERAL** - The site was previously occupied by a large house built in

**STORM WATER** - The existing site is a brown field site being previously occupied and measures some 0.11 ha in area. it is located in flood zone 1 area for fluvial flooding and storm water flooding. No flood risk assessment was required for this project.

The proposed site offer the following areas:-

Site boundary area - 2200 sq.m
Roofs - (5 x 78 sq.m) = 390 sq.m
External permeable road - 368 sq.m

The existing ground offers good porosity tests which were carried out by the client offering an average design rate of $1.27 \times 10^{-6}$ m/s with a factor of safety of 1.5 to offers a required infiltration pavement depth of 240mm solution beneath the private road using a open graded asphalt and infiltration gravel blanket to discharge the site storm water. The CBR value will determine if a further capping layer is required.

This road pavement will also receive the rainwater flows from the roof rainwater butts and threshold drainage channels. Flow from the water butts allow roof water to then enter a a catch/silt pit which provides settlement. The outlet pipe then connects to the piped below ground system to the final infiltration soakaway beneath the private road. The road surface will be open graded asphalt and this will need to be kept clean to ensure waterways are maintained.

The road infiltration storage volume is flat at finished and formation levels thus ensuring that water retention is maximised in the volume available with no high areas of pavement.

Around the houses is provided an infiltration permeable pavement which drains itself to the ground - No additional flows from roofs or other areas are added to this, therefore there is no concentration of flows - the pavement drains only its own area.

All infiltration volumes have been designed to achieve sufficient storage for a 100 year +40% climate change storm.

The attenuated storage will infiltrate to the ground and it is important to note that the construction requires a geotextile beneath to separate formation and stone layers - Refer to SDA detail no.
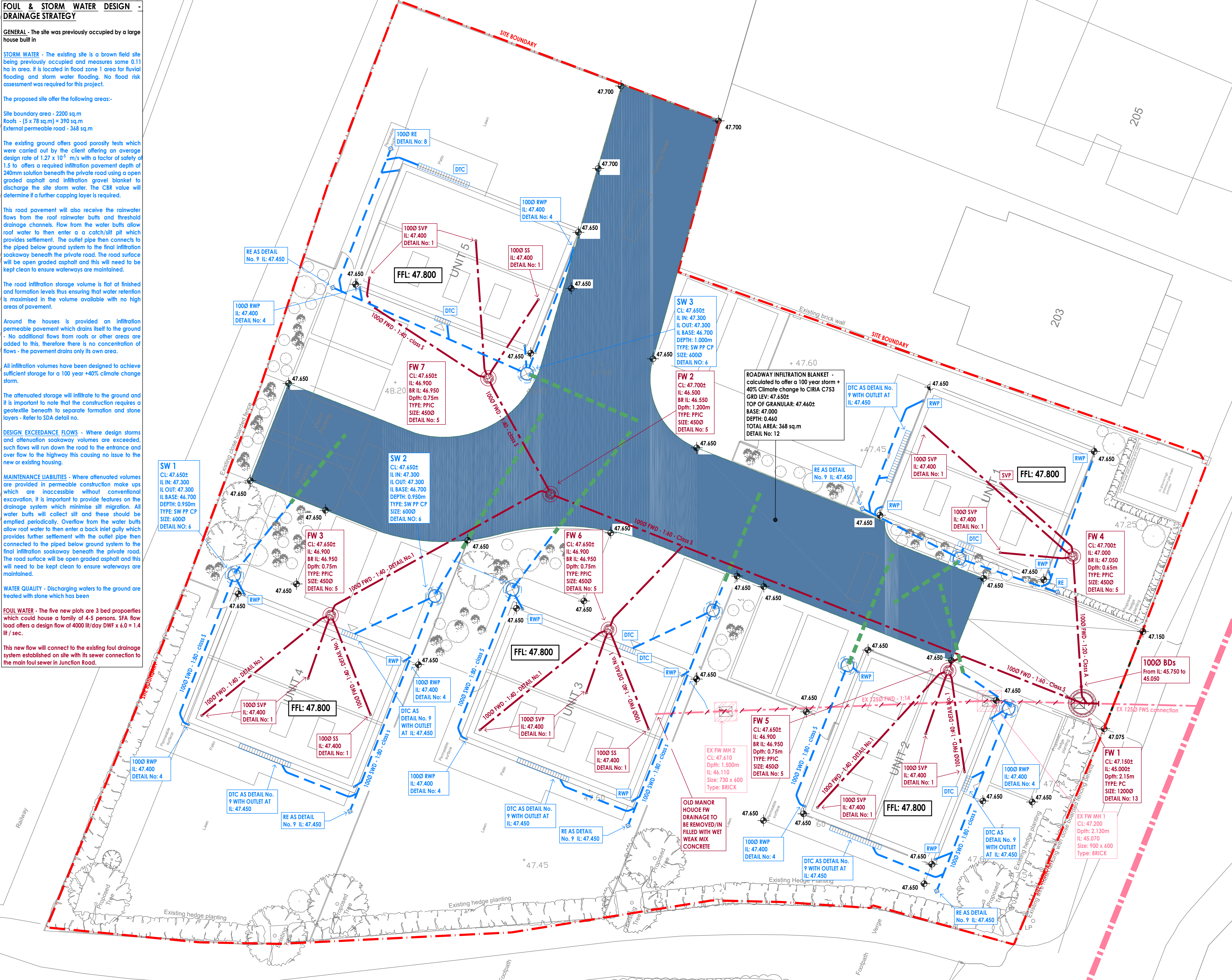
**DESIGN EXCEEDANCE FLOWS** - Where design storms and attenuation soakaway volumes are exceeded, such flows will run down the road to the entrance and over flow to the highway this causing no issue to the new or existing housing.

**MAINTENANCE LIABILITIES** - Where attenuated volumes are provided in permeable construction make ups which are inaccessible without conventional excavation, it is important to provide features on the drainage system which minimise silt migration. All water butts will collect silt and these should be emptied periodically. Overflow from the water butts allow roof water to then enter a back inlet gully which provides further settlement with the outlet pipe then connected to the piped below ground system to the final infiltration soakaway beneath the private road. The road surface will be open graded asphalt and this will need to be kept clean to ensure waterways are maintained.

**WATER QUALITY** - Discharging waters to the ground are treated with stone which has been

**FOUL WATER** - The five new plots are 3 bed properties which could house a family of 4-5 persons. SFA flow load offers a design flow of 4000 lit/day DWF x 6.0 = 1.4 lit / sec.

This new flow will connect to the existing foul drainage system established on site with its sewer connection to the main foul sewer in Junction Road.

THIS DRAWING IS SUBJECT TO:-
- EXISTING FOUNDATIONS
- APPROVAL BY THE PLANNERS
- APPROVAL BY BUILDING CONTROL

| Rev | Description | Date | By |
|---|---|---|---|
| P5 | Porosity rate typo corrected. | 10.10.2025 | SAD |
| P4 | Drainage altered to suit 5 nr house application. | 22.08.2025 | SAD |
| P3 | PLANNING ISSUE | 11.12.2019 | SAD |
| P2 | Soakaways removed and drainage altered to suit the 10m restriction distance by Network Rail. | 25.10.2019 | SAD |
| P1 | PRELIMINARY ISSUE | 11.10.2019 | SAD |

**CLIENT:**
Mrs K Safronova

**PROJECT:**
201 Junction Road Burgess Hill
West Sussex RH15 0NU

**TITLE:**
Site Plan - Proposed Foul and Storm Water Drainage

**SCALE:**
GA 1:100 @ A1
GA 1:200 @ A3

**DRAWING STATUS:**
FOR PLANNING

**JOB No:** 1593
**DWG No & Rev:** 100 P5

**SIMON DENT ASSOCIATES**
Public Health, Civil and Water Consulting Engineers

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West Sussex BN43 6PJ

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0754 658 3150

## Drainage schedule labels

**SW 3**
CL: 47.650±
IL IN: 47.300
IL OUT: 47.300
IL BASE: 46.700
DEPTH: 1.000m
TYPE: SW PP CP
SIZE: 600Ø
DETAIL NO: 6

**SW 2**
CL: 47.650±
IL IN: 47.300
IL OUT: 47.300
IL BASE: 46.700
DEPTH: 0.950m
TYPE: SW PP CP
SIZE: 600Ø
DETAIL No: 6

**SW 1**
CL: 47.650±
IL IN: 47.300
IL OUT: 47.300
IL BASE: 46.700
DEPTH: 0.950m
TYPE: SW PP CP
SIZE: 600Ø
DETAIL NO: 6

**FW 7**
CL: 47.650±
IL: 46.900
BR IL: 46.550
Dpth: 0.75m
TYPE: PPIC
SIZE: 450Ø
DETAIL No: 5

**FW 2**
CL: 47.700±
IL: 46.500
BR IL: 46.550
Dpth: 1.200m
TYPE: PPIC
SIZE: 450Ø
DETAIL No: 5

**FW 3**
CL: 47.650±
IL: 46.900
BR IL: 46.950
Dpth: 0.75m
TYPE: PPIC
SIZE: 450Ø
DETAIL No: 5

**FW 6**
CL: 47.650±
IL: 46.900
BR IL: 46.950
Dpth: 0.75m
TYPE: PPIC
SIZE: 450Ø
DETAIL No: 5

**FW 4**
CL: 47.700±
IL: 47.000
BR IL: 47.050
Dpth: 0.65m
TYPE: PPIC
SIZE: 450Ø
DETAIL No: 5

**FW 5**
CL: 47.650±
IL: 46.900
BR IL: 46.950
Dpth: 0.75m
TYPE: PPIC
SIZE: 450Ø
DETAIL No: 5

**FW 1**
CL: 47.150±
IL: 45.000±
Dpth: 2.15m
TYPE: PC
SIZE: 1200Ø
DETAIL No: 13

**ROADWAY INFILTRATION BLANKET** - calculated to offer a 100 year storm + 40% Climate change to CIRIA C753
GRD LEV: 47.650±
TOP OF GRANULAR: 47.460±
BASE: 47.000
DEPTH: 0.460
TOTAL AREA: 368 sq.m
DETAIL No: 12

**EX FW MH 2**
CL: 47.610
Dpth: 1.500m
IL: 46.110
Size: 730 x 600
Type: BRICK

**EX FW MH 1**
CL: 47.200
Dpth: 2.130m
IL: 45.070
Size: 900 x 600
Type: BRICK

**100Ø BDs**
From IL: 45.750 to 45.050

OLD MANOR HOUSE FW DRAINAGE TO BE REMOVED/IN FILLED WITH WET WEAK MIX CONCRETE

FFL: 47.800