SuDS & Foul Drainage Statement

75 Folders Lane, Burgess Hill, RH15 0DY

Client

Talbot Developments (Sussex) Ltd

Ref: 13884

Date: November 2025

Consulting Engineers

GTA Civils & Transport Ltd
Maple House
192 – 198 London Road
Burgess Hill
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Schedule of Appendices

Issue	Issue date	Compiled	Checked
Preliminary/ 1st Issues	11 November 2025	JP	FVV / MR

1 Introduction

1.1 GTA Civils & Transport Ltd. was appointed by the Client to prepare a SuDS & Foul Drainage Statement in relation to the proposed development at 75 Folders Lane, Burgess Hill, RH15 0DY. No responsibility is accepted to any third party for all or part of this study in connection with this or any other development.

1.2 The objective of this Statement is to support a planning application to Mid Sussex District Council and demonstrate that the foul and surface water drainage networks shall comply with current national and local drainage policies.

2 Existing Site

2.1 The application site comprises a redundant clinic building, accessed from Folders Lane to the south. It lies in the area administered by Mid Sussex District Council (MSDC). A site location map and an aerial view are shown in Appendix A.

2.2 Hydrology: the site falls within the River Adur's catchment. The Adur is the nearest hydrological feature and is a Main River at this point. It flows northwards approximately 220m to the west at its closest.

2.3 Topography: a topographic survey is included in Appendix B. The site is relatively flat/level with slopes levels ranging from 53.10m Above Ordnance Datum (AOD) to 52.10m AOD approximately at the NW corner. Levels around the building are typically 52.5m – 52.6m AOD.

2.4 Geology: The BGS's online geology map shows the area is underlain by Weald Clay (mudstone) with no recorded superficial deposits. From experience of many residential developments in recent years along Folders Lane, it is clear that this soil type possesses too low a soakage rate for infiltration features, eg soakaways and porous pavings.

2.5 Public sewers: Southern Water's records (see Appendix B) show a 150mm diameter foul sewer flowing westwards along Folders Lane, with manhole ref 7002 closest to the site's access. There are no surface water public sewers serving this part of Burgess Hill. The depth to the foul sewer is 5.36m.

2.6 On-site drainage: the topographic survey (in Appendix B) shows the foul and storm drainage networks. The surface water drains are routed with the foul and this combined network is routed south – to the public sewer (subject to a drainage survey). This is commonplace in Burgess Hill because of there being no public storm sewers and the Clay soil type offering insufficient soakage potential. There is no indication that the flow rate is restricted into the public sewerage.

2.7 Greenfield runoff rate calculations were generated using the HR Wallingford online application (UK SuDS) – refer to Appendix D. The Qbar rate for the contributing area of 0.08ha is 1.0l/s.

3 Proposed SuDS Strategy and Foul Drainage

3.1 This proposal comprises building 4 new dwelling houses (2x detached and 2x semi-detached) after the redundant clinic building is demolished.

3.2 The NPPF states that SuDS should be utilised unless there are practical reasons for not doing so. The SuDS hierarchy has been applied, with infiltration features at the top of this list. A drainage strategy layout has been developed, applying the SuDS Hierarchy (infiltration, watercourses, public sewers). It is proposed to route the site's runoff to Southern Water's sewer, as per the current set up – refer to section 2.6 above. (The clay soil precludes the use of infiltration features and there is no watercourse within this site.) All other possible routes have been examined and determined to be unfeasible.

3.3 The drainage strategy layout, in Appendix C, shows the development's surface water routed via a porous paved area to the existing public sewer to the south.

3.4 There is no surface water sewer available and so an indirect connection is proposed into the foul sewer – subject to Southern Water's approval. It is contended that there will be a significant improvement compared to the existing setup: the offsite flow rate into the sewer shall be restricted to 1l/s while it is unrestricted at present.

3.5 Calculations are included in Appendix D, using FEH22 rainfall data and Cv values of 1. These demonstrate that the paved area shall contain all storm events up to and including the 1 in 100+45% climate change event.

3.6 Any exceedance flows would drain overland west-northwest, much as per the existing situation, with no increase in flood risks downstream.

3.7 The current 2025 National Standards for SuDS published by DEFRA were applied. A water butt shall be installed in each unit's garden - standard 1 is met in this manner. Unlined porous pavings satisfy Standard 2 by intercepting the first 5mm of each rainfall event (because the area of pavings exceeds the impermeable area.)

3.8 Water quality: the proposed permeable driveway will provide pollutant controls for the driveway runoff; with the pollution hazard of the roofs expected to be very low. This is in line with the Simple Index approach as described in the SuDS manual.

3.9 It is proposed to route the new units' foul effluent to the public sewer, via the existing connection between 7002 and 7001 – refer to the sewer records in Appendix B. Note that this connection is to be

confirmed by undertaking a CCTV drainage survey at the next stage. This indirect connection will be subject to a S106 application in due course to Southern Water.

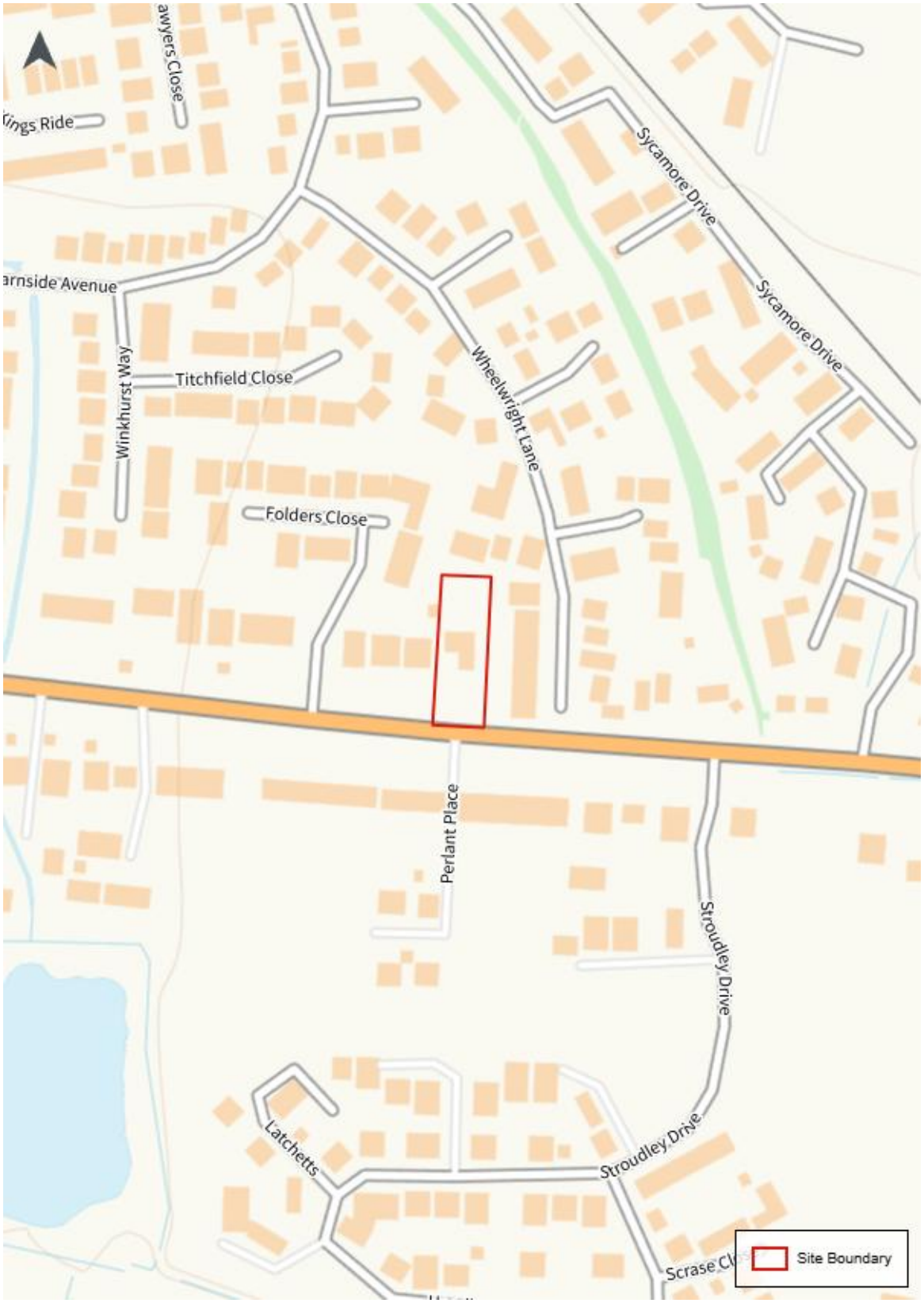
3.10 Maintenance: A planning stage drainage maintenance schedule is included in Appendix E. This sets out how the various components will be inspected and maintained – in line with Industry Standards as per CIRIA's SuDS Manual C753. The responsibility of maintenance, repair, renewal and replacement of the surface water drainage system will be conferred on to the property owners with all costs shared on an equal basis. These arrangements will be administered in the Transfer Document **TP1** of each property at the point of sale with the same responsibilities transferring with each property to the successors in title.

3.11 It is contended that the development's SuDS and foul drainage networks shall be fully sustainable and comply fully with national and local drainage policies.

- End of Statement -

Appendix A

Site Location Map & Aerial Photo




Site Boundary

Appendix B

Topographic Survey & Sewer Records

LAYER INFORMATION

LINETYPES & SYMBOLS

BUILDING OUTLINE
KERB LINE
FENCE
SURFACE EDGE
FOUL DRAINAGE
STORM DRAINAGE
OVERHEAD CABLES
FOLIAGE OUTLINE
SURVEY STATION
GATES
TREE CANOPY
SLOPE SYMBOL

LEGEND

DRAINAGE LEGEND

NORTH (INDICATIVE)

M.J.ZARA ASSOCIATES
LAND & MEASURED BUILDING SURVEYORS
139 HIGH STREET – LEWES
EAST SUSSEX – BN7 1XS
TEL 01273 472492
E-MAIL mjzaraassociates@gmail.com

SITE
75 FOLDERS LANE "FINCHES"
BURGESS HILL
WEST SUSSEX

CLIENT
TALBOT DEVELOPMENTS (SUSSEX) LTD
ORD
KINGSTON ROAD
LEWES
EAST SUSSEX
BN7 3NB

NOTES
LEVELS AND COORDINATES BASED ON ORDNANCE SURVEY BY G.P.S.

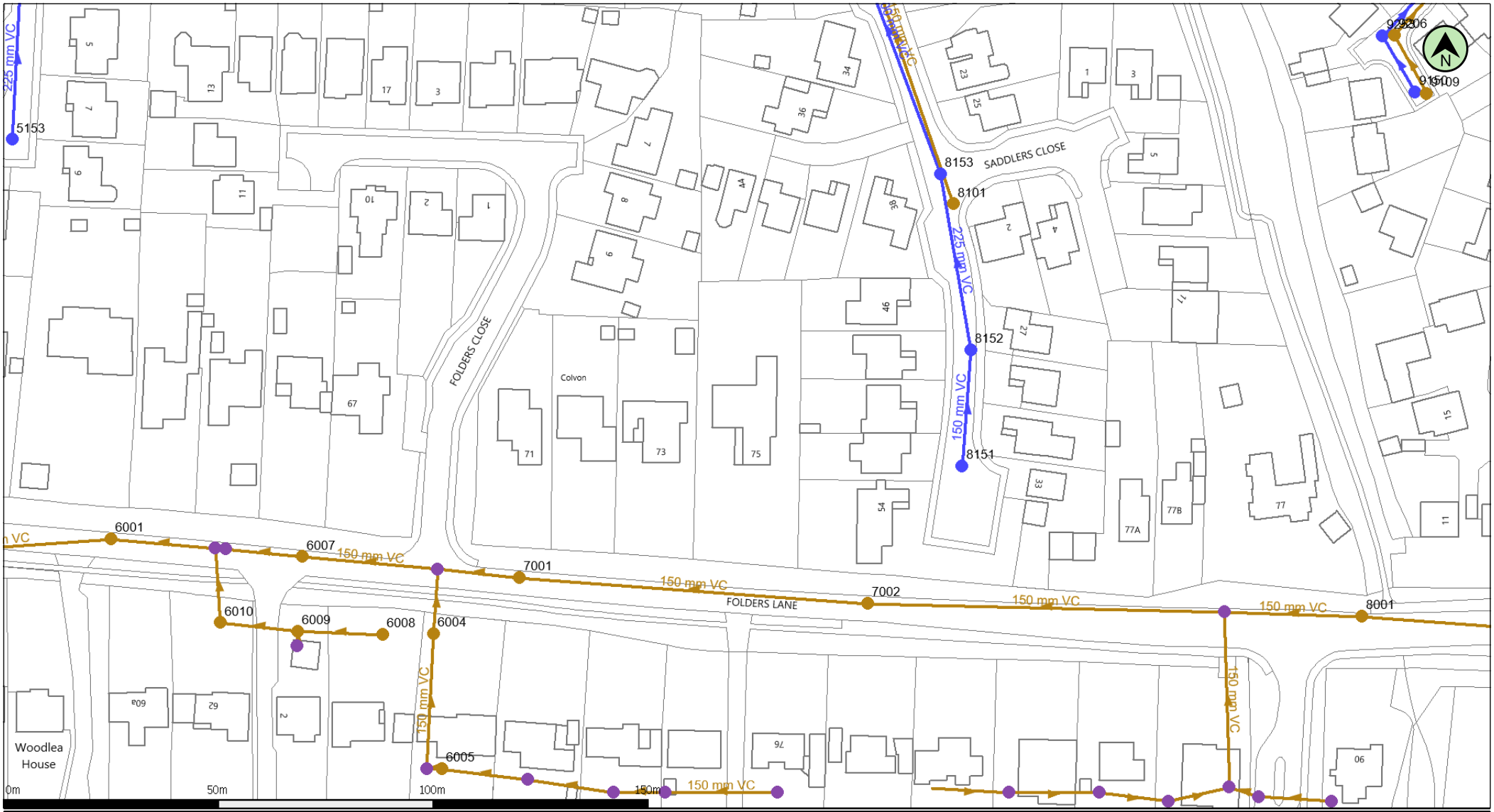
TREE HEIGHTS ARE APPROXIMATE

DRAINAGE TYPES ASSESSED BY VISUAL INSPECTION ONLY.
CONNECTIONS ARE INDICATIVE ONLY.
SPECIALIST CONTRACTORS SHOULD BE SOUGHT TO ESTABLISH EXACT PIPE RUNS AND TYPES.

PLEASE DO NOT SCALE OFF A PRINTED PLAN.

JOB REF. 9959

SCALE	SHEET SIZE
1:100	A0 PORTRAIT
DATE	SURVEYED BY
JULY 2025	CS



(c) Crown copyright and database rights 2025 Ordnance Survey AC0000808122 | Date: 22/09/25 | Scale: 1:1250 | Map Centre: 532755,118121 | Data updated: 21/08/25 | Our Ref: 1886303 - 1 | Wastewater Plan A4 Powered by digdat

The positions of pipes shown on this plan are believed to be correct, but Southern Water Services Ltd accept no responsibility in the event of inaccuracy. The actual positions should be determined on site. This plan is produced by Southern Water Services Ltd (c) Crown copyright and database rights 2025 Ordnance Survey AC0000808122 .This map is to be used for the purposes of viewing the location of Southern Water plant only. Any other uses of the map data or further copies is not permitted.

WARNING: BAC pipes are constructed of Bonded Asbestos Cement.

WARNING: Unknown (UNK) materials may include Bonded Asbestos Cement.

Foul Gravity Sewer
Combined Gravity Sewer
Culverted Water Course or Treated Effluent
Surface Water Gravity Sewer
Combined Pumping Station
Surface Water Pumping Station
Foul Pumping Station
Foul Manhole
Combined Manhole
Surface Water Manhole
Side Entry Manhole, Demarcation Chamber, Dummy Manhole or Surface Water Soakaway
Rising Main, Vacuum or Syphon
Combined Outfall
Surface Water Outfall
Foul Outfall
Surface Water Inlet
Water Treatment Works
Section 104 Area
Building Over Agreement Area

cfagan@gtacivils.co.uk

13884 75 Folders Ln

WATER for LIFE from Southern Water.

Manhole Reference	Liquid Type	Cover Level	Invert Level	Depth to Invert
6001	F	49.72	0.00	-
6004	F	48.27	48.27	-
6005	F	51.94	0.00	-
6007	F	0.00	0.00	-
6008	F	0.00	0.00	-
6009	F	0.00	0.00	-
6010	F	0.00	0.00	-
7001	F	51.82	47.18	-
7002	F	53.46	48.10	-
8001	F	52.46	49.03	-
8101	F	52.53	50.69	-
9109	F	51.23	49.00	-
9206	F	50.90	48.76	-
5153	S	47.77	0.00	-
8151	S	52.91	51.94	-
8152	S	52.78	51.49	-
8153	S	52.55	51.01	-
9150	S	51.20	49.85	-
9253	S	51.08	49.51	-

Manhole Reference	Liquid Type	Cover Level	Invert Level	Depth to Invert

Appendix D

Drainage Calculation Sheets

GENERAL NOTES

1. The location, size, depth and identification of existing services that may be shown or referred to on this drawing have been assessed from non intrusive observations , record drawings or the like. The contractor shall safely carry out intrusive investigations, trial holes or soundings prior to commencing work to satisfy himself that it is safe to proceed and that the assessments are accurate. any discrepancies shall be notified to gta prior to works commencing.

2. Tender or billing drawings shall not be used for construction or the ordering of materials.

3. Do not scale. All dimensions and levels to be site confirmed.

4. This drawing shall be read in conjunction with all relevant architects, consultants drawings and specifications, together with H&S plan requirements.

5. Copyright : This drawing must not be copied, amended nor reproduced without the prior written agreement of gta.

6. All drawings specifications and recommendations made by gta are subject to Local Authority and other relevant Statutory Authorities approval. Any works or services made abortive due to the client proceeding prior to these approvals is considered wholly at the Clients risk. gta hold no responsibility for resulting abortive works or costs.

7. If viewing this drawing as an Autocad file (.dwg) in digital format then it is done so with this Disclaimer due to the fact that it can be altered and manipulated following its issue by GTA Civils & Transport and therefore, any alteration or modification of DWG data files provided by GTA Civils & Transport, by you or a third party, without GTA Civils and Transport's express written approval, is done so entirely at your own risk. Modification includes (but is not limited to) turning layers on and off, unfreezing layers and reloading, turning on and off print functions and unloading x-refs.

8. Your attention is also drawn to the fact that the information contained within this file may be subject to alteration at any time, pending technical approval from an approving authority or at the client's instruction. It is therefore strongly recommended that multiple and regular cross checks are made against the current contract drawings.
It is your responsibility to ensure that the correct issue or revision of the DWG data file is being used and requests for updated information made accordingly.

9. Should any apparent discrepancies between the data contained within the DWG file and the current contract drawings become evident, it must be reported back to GTA Civils & Transport as soon as reasonably practicable. Precedence should be given to the current contract drawings (PDF) unless advised otherwise.

KEY

- Private surface water drain
- Private foul drain
- FIC — Foul Plastic Inspection Chamber
- SIC — Surface Water Plastic Inspection Chamber
- SAC — 300mm diameter Surface Water Access Chamber
- FAC — 300mm diameter Foul Water Access Chamber
- DT — Distribution/ collection tank in permeable paving sub-base
- Permeable Paving area
- Combined SW&FW Inspection Chamber
- Overland Flow Arrow

UNLINED SUBBASE STORAGE AREA S6
Flat Formation
Finished ground level 52.400
Minimum 625mm depth
Porosity 0.3
Min base IL 51.695

FIC
CL 52.392
IL 51.160

FWD 100Ø 5.87m 1:39

FIC
CL 52.437
IL 51.350

RWP

RWP
FWD 100Ø 10.83m 1:33

FIC
CL 52.520
IL 51.600

DT

SIC

SIC

DT

FWD 100Ø 13.36m 1:74

DT

DT

SIC

SIC

FIC
CL 52.437
IL 51.350

FWD 100Ø 8.98m 1:39

FIC
CL 52.530
IL 51.600

FWD 100Ø 7.94m 1:33

ExFIC

FIC
CL 52.445
IL 51.020

RWP

RWP

DT

DT

S6 (STORAGE)
CL 52.479
IL 50.990

FWD 100Ø 14.06m 1:74

SWD 300Ø 16.64m 1:36

FIC
CL 52.620
IL 50.53

FWD 100Ø 16.74m 1:56

FIC
CL 52.612
IL 50.830

S3
CL 52.644
IL 50.530

SWD 300Ø 13.37m 1:17

S4
CL 52.637
IL 49.536

S7
1200Ø
CL 52.649
IL 49.850

Existing FW sewer to be grubbed up to new connection point

New connection via chamber on existing foul run. Levels to be confirmed prior to start of works. Subject to new S106 Indirect Connection application

Existing FW drainage route interpolated - to be surveyed prior to works

Flow rate restricted to 1.1 l/s via Hydrobrake with non-return valve on outlet

Refuse kerbside collection point

FOLDERS LANE

73

0 1m 2m 3m 4m 5m 10m

Rev	Amendments	Date	Dsn	Chk
P2	SW Drainage configuration adjusted	10/11/25	NG	FVV
P1	INITIAL ISSUE	06/11/25	NG	FVV

Status: PRELIMINARY

Client: TALBOT DEVELOPMENTS

Architect:

Project: 75 FOLDERS LANE
BURGESS HILL

Title: DRAINAGE STRATEGY

Date: OCTOBER 2025

Scale @ A1: 1:100

Clients Ref.:

Project Ref.: 13884

gta Civils & Transport
Maple House, 192-198 London Road,
Burgess Hill, West Sussex, RH15 9RD
Tel.01444 871444 Web: www.gtacivils.co.uk

Drawing Number: 13884-1600

Rev.: P2

Appendix C

Proposed Drainage Strategy Layout

Greenfield runoff rate estimation tool

www.uksuds.com | Greenfield runoff rate estimation tool (https://www.uksuds.com/)

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	10/11/2025
Calculated by	NG
Reference	13884
Model version	2.2.2

Location

Site name	75 Folders Lane
Site location	Burgess Hill

Site easting (British National Grid)	532847
Site northing (British National Grid)	118109

Site details

Total site area (ha)	0.09012 ha

Greenfield runoff

Method

Method	FEH statistical (2025)

FEH statistical (2025)

	My value		Map value
SAAR9120 (mm)	911 mm		
BFIHOST19scaled	0.236		
QMed-QBar conversion	1.136		1.136
QMed (l/s)	0.8 l/s		
QBar (FEH statistical 2025) (l/s)	1 l/s		

Growth curve factors

	My value		Map value
Hydrological region	7		7
1 year growth factor	0.85		
2 year growth factor	0.88		
10 year growth factor	1.62		
30 year growth factor	2.3		
100 year growth factor	3.19		
200 year growth factor	3.74		

Results

Method	FEH statistical (2025)
Flow rate 1 year (l/s)	0.8 l/s
Flow rate 2 year (l/s)	0.8 l/s
Flow rate 10 years (l/s)	1.6 l/s
Flow rate 30 years (l/s)	2.2 l/s
Flow rate 100 years (l/s)	3.1 l/s
Flow rate 200 years (l/s)	3.6 l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent 'zero' figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)	Invert Level (m)
S6	0.071	5.00	52.480		532746.121	118111.843	1.490	50.990
S3	0.008	5.00	52.644	450	532746.116	118095.130	2.114	50.530
S7 (hydrobrake)			52.649	1200	532759.200	118094.993	2.799	49.850
S4			52.637	450	532768.848	118095.124	3.101	49.536

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	S6	S3	16.644	0.600	50.990	50.530	0.460	36.2	300	5.11	5.0
1.001	S3	S7 (hydrobrake)	13.084	0.600	50.530	49.850	0.680	19.2	300	5.17	5.0
1.002	S7 (hydrobrake)	S4	3.911	0.600	49.850	49.536	0.314	12.5	300	5.18	5.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	2.622	185.3	1.3	1.190	1.814	0.071	0.0	18	0.765
1.001	3.600	254.5	1.4	1.814	2.499	0.079	0.0	16	0.984
1.002	4.478	316.5	1.4	2.499	2.801	0.079	0.0	14	1.134

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Normal	Starting Level (m)	
Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s)	x
Summer CV	1.000	Drain Down Time (mins)	240	Check Discharge Volume	x
Winter CV	1.000	Additional Storage (m³/ha)	0.0		

Storm Durations

15	30	60	120	180	240	360	480	600	720	960	1440

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	0	0
30	0	0	0
100	45	0	0

Node S7 (hydrobrake) Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	x	Sump Available	✓
Invert Level (m)	49.850	Product Number	CTL-SHE-0040-6000-0625-6000
Design Depth (m)	0.625	Min Outlet Diameter (m)	0.075
Design Flow (l/s)	0.6	Min Node Diameter (mm)	1200

Node S6 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	51.695	Slope (1:X)	999.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)		Depth (m)	0.625
Safety Factor	2.0	Width (m)	27.500	Inf Depth (m)	
Porosity	0.30	Length (m)	10.000		

Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
240 minute summer	S6	168	51.750	0.760	4.6	4.1090	0.0000	SURCHARGED
240 minute summer	S3	160	51.750	1.220	4.1	0.1940	0.0000	SURCHARGED
240 minute summer	S7 (hydrobrake)	160	51.750	1.900	2.7	2.1494	0.0000	SURCHARGED
240 minute summer	S4	172	49.548	0.012	1.0	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
240 minute summer	S6	1.000	S3	3.5	0.854	0.019	1.1721	
240 minute summer	S3	1.001	S7 (hydrobrake)	2.7	0.259	0.011	0.9214	
240 minute summer	S7 (hydrobrake)	1.002	S4	1.0	1.007	0.003	0.0038	19.6

Results for 30 year Critical Storm Duration. Lowest mass balance: 93.25%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
180 minute winter	S6	176	51.955	0.965	8.0	21.0012	0.0000	SURCHARGED
180 minute winter	S3	176	51.955	1.425	3.2	0.2265	0.0000	SURCHARGED
180 minute winter	S7 (hydrobrake)	176	51.955	2.105	2.0	2.3805	0.0000	SURCHARGED
180 minute winter	S4	176	49.549	0.013	1.0	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
180 minute winter	S6	1.000	S3	2.7	0.815	0.015	1.1721	
180 minute winter	S3	1.001	S7 (hydrobrake)	2.0	0.306	0.008	0.9214	
180 minute winter	S7 (hydrobrake)	1.002	S4	1.0	1.020	0.003	0.0039	23.5

Results for 100 year +45% CC Critical Storm Duration. Lowest mass balance: 93.67%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
360 minute winter	S6	344	52.374	1.384	8.2	51.1662	0.0000	FLOOD RISK
360 minute winter	S3	344	52.374	1.844	2.3	0.2931	0.0000	FLOOD RISK
360 minute winter	S7 (hydrobrake)	344	52.373	2.523	1.7	2.8539	0.0000	FLOOD RISK
360 minute winter	S4	344	49.549	0.013	1.1	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
360 minute winter	S6	1.000	S3	2.0	0.815	0.011	1.1721	
360 minute winter	S3	1.001	S7 (hydrobrake)	1.7	0.149	0.007	0.9214	
360 minute winter	S7 (hydrobrake)	1.002	S4	1.1	1.042	0.004	0.0042	35.7

Appendix E

Draft SuDS Maintenance Plan

1. Introduction

This Plan sets out the framework for the management of the proposed sustainable drainage systems (SuDS) and foul drainage network. The document will be updated with further information through the next stage of detailed design and coordination. At this stage, what is set out herein is intended to be sufficient to demonstrate the viability of the proposed SuDS and foul drainage maintenance regime for planning purposes.

2. Ownership & Maintenance Responsibilities

It is the overarching responsibility of each **site owner** to ensure the Drainage Infrastructure is maintained in accordance with this Maintenance Plan.

The responsibility of maintenance, repair, renewal and replacement of the surface water drainage system will be conferred on to the property owners with all costs shared on an equal basis. These arrangements will be administered in the Transfer Document TP1 of each property at the point of sale with the same responsibilities transferring with each property to the successors in title.

Key elements of the drainage system which require maintenance are:

- Porous Pavings
- Pipes and Chambers

The following sections show schedules detailing the maintenance requirements for each of the main drainage items used within the scheme. The Owner will undertake the inspections and maintenance activities in accordance with these schedules or engage with specialist maintenance contractors to undertake maintenance tasks on their behalf.

Additional reference has been made to currently established best practice and guidance documents such as The SuDS Manual (CIRIA C753, 2015) and other resources available at the susdrain website.

This Plan is to be a live document. The frequency of maintenance intervals may need to be increased or decreased based on the observed performance of the drainage systems over time.

3. Health and Safety

All those responsible for and involved in the maintenance of the site drainage systems should be safety-conscious and comply with the relevant health and safety legislation. This includes:

- The Health and Safety at Work etc Act 1974
- The Management of Health and Safety at Work Regulations 1999
- The Workplace (Health, Safety and Welfare) Regulations 1992

The Employer of maintenance operatives is responsible for suitable risk assessment and management to ensure safe working conditions and practices. Measures to protect potential visitors must also be drawn up.

Specialist contractors used should work to industry guidelines and be able to demonstrate safe working practices.

Employers have a duty to employees to inform them about the risks of their work environment and to decrease the risk as far as reasonably practicable. Appropriate personal protective equipment (PPE) should be provided and practices/policies implemented based on risk assessments.

Operatives should be trained for working near water. Risks of contaminated water should be considered. Checking for open cuts and using nitrile gloves, waterproof plasters etc is advised.

Entry of pipes, chambers and culverts should be avoided. Work should be carried out from the surface using appropriate equipment. In the event that entry cannot be avoided to perform a critical task, the required safety training, protection measures and precautions must be implemented prior to entry. Lone working should never be attempted.

For further information refer to Section 36 of The SuDS Manual (CIRIA C753).

4. Schedule A – Sewers, Manholes, Gullies, Channel Drains

Regular inspection and maintenance are required to ensure the effective long-term operation of private drains, manholes, gullies & channel drains.

All gullies and drainage are private and to be maintained by the homeowner. Operation and maintenance requirements for all sewers, manholes, gullies and channel drains are described in the following table:

Schedule	Action	Frequency
Regular Maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	6 Monthly intervals
	Common yard & car park & other hard standing areas to be swept clear of debris, to prevent possibility of blockages to the receiving drainage systems	Monthly
	Debris removal from gullies & channel drains (where may cause risks to performance)	Monthly intervals, after autumn leaf fall, or as required based on specific observations
	Lift and inspect receiving manholes to check for any blockages	Monthly
Remedial Actions	Repair any damaged gully or channel drain gratings.	As required
	Replace / fix any loose channel drain covers	As required

Where appropriate refer also to specialist drainage manufacturer's information and maintenance requirements.

In all instances, inspection and cleaning should be carried out only by a specialist contractor and in accordance with the guidelines given in `Safe Working in Sewers and at Sewage Works' published by National Joint Health and Safety Committee for the Water Services.

5. Schedule B – Porous Pavings

Inspection Frequency and Maintenance Requirements: as per table below.

Schedule	Action	Frequency
Regular Maintenance	Inspect for sediment and debris in pre-treatment components	Annually
	Cleaning of gutters and any filters on downpipes	Annually (or as required based on inspections)
	Trimming any roots that may be causing blockages	Annually (or as required based on inspections)
Occasional Maintenance	Removal of sediment from pre-treatment devices (silt trap/catchpit)	Six monthly
	Remove tree roots or trees that are close to the blanket	As required
Remedial Actions	Rehabilitate infiltration or filtration surfaces	As required
	Inspect surface for clogging, standing water and structural damage	Monthly
Monitoring	Inspect pre-treatment devices (silt trap/ catchpit) and ground surface for silt accumulation. Establish appropriate silt removal frequencies	Half-yearly

Sediments excavated from upstream pre-treatment devices that receive runoff from residential roof areas are generally not toxic or hazardous material and can be safely disposed of by either land application or landfilling. However, consultation should take place with the environmental regulator to confirm appropriate protocols. Sediment testing may be required before sediment excavation to determine its classification and appropriate disposal method.

6. Contamination or Dilution of Spillage

In the event of a spillage, it is the responsibility of the landowner to clear up any spillage before it enters the drainage system. The primary method of dealing with any spillage of hydrocarbons should be using sand to soak up the leak and prevent any hydrocarbons entering the drainage system. Once sand has been contaminated it should not be washed into the drainage system but disposed of by a Licensed Contractor.

Environment Agency – Emergency Contact Number

In the event of a spillage the Environment Agency should be contacted to notify the event and seek advice. The Environment Agency Incident Hotline is **0800 80 70 60** (Freephone 24hrs).

Civil Engineering - Transport Planning - Flood Risk

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