

Chris & Sarah Mellish.

MONSON

Structural Engineering
Roads & Car Parks
Traffic & Flood Risk Assessments
Water & Drainage Engineering
Technical Audits & Assessments

**North Hall Cottage
Mallion's Lane
Staplefield
RH17 6AT**

Proposed Residential Extension

**Surface Water Drainage
Strategy**

Issue	B
Date	09 April 2025
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Verified by	M. Wojcieszak
Job No	25127



Registered No. FS 37624

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APPENDICES

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Appendix B – Topographical Survey

Appendix C – Site Geology

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Appendix E – Surface Water Drainage Proposal

1.00 Introduction

- 1.01 Monson Engineering has been instructed to prepare a Surface Water Drainage Strategy to support a planning application for the residential extension and internal alterations at the North Hall Cottage, Mallion's Lane, Staplefield, West Sussex, RH17 6AT.
- 1.02 The proposal is for the internal alterations and extension to facilitate a kitchen, dining, and family bathroom to the east of the North Hall Cottage.
- 1.03 The approximate National Grid Reference of the site is TQ 27158 26935 (527158E, 126935N). A site location plan and development proposals can be found in **Appendix A**.

2.00 Existing Drainage

- 2.01 The topographical map indicates the general fall of the ground from the south to the north. Any stormwater runoff will either evaporate, infiltrate where possible, or follow this topography.
- 2.02 The Mapinfo record shows there is no public sewer in the vicinity of the proposed properties. The foul sewer of the existing cottage is disposed of on-site via a cesspit and filter bed.
- 2.03 Topographical MapInfo records showing existing sewer arrangement records are included in **Appendix B**.

3.00 Site Geology

- 3.01 The British Geological Survey (BGS) shows that the application site is underlain by the Weald Clay Formation-Mudstone. The BGS extract is included in **Appendix C**.
- 3.02 The bedrock formation of the Weald Clay is distinguished by dark grey, thinly bedded mudstones (shales) and mudstones with subordinate siltstones, fine- to medium-grained sandstones, including calcareous sandstone (e.g. Horsham Stone Member), shelly limestones (the so-called "Paludina Limestones") and clay ironstones.

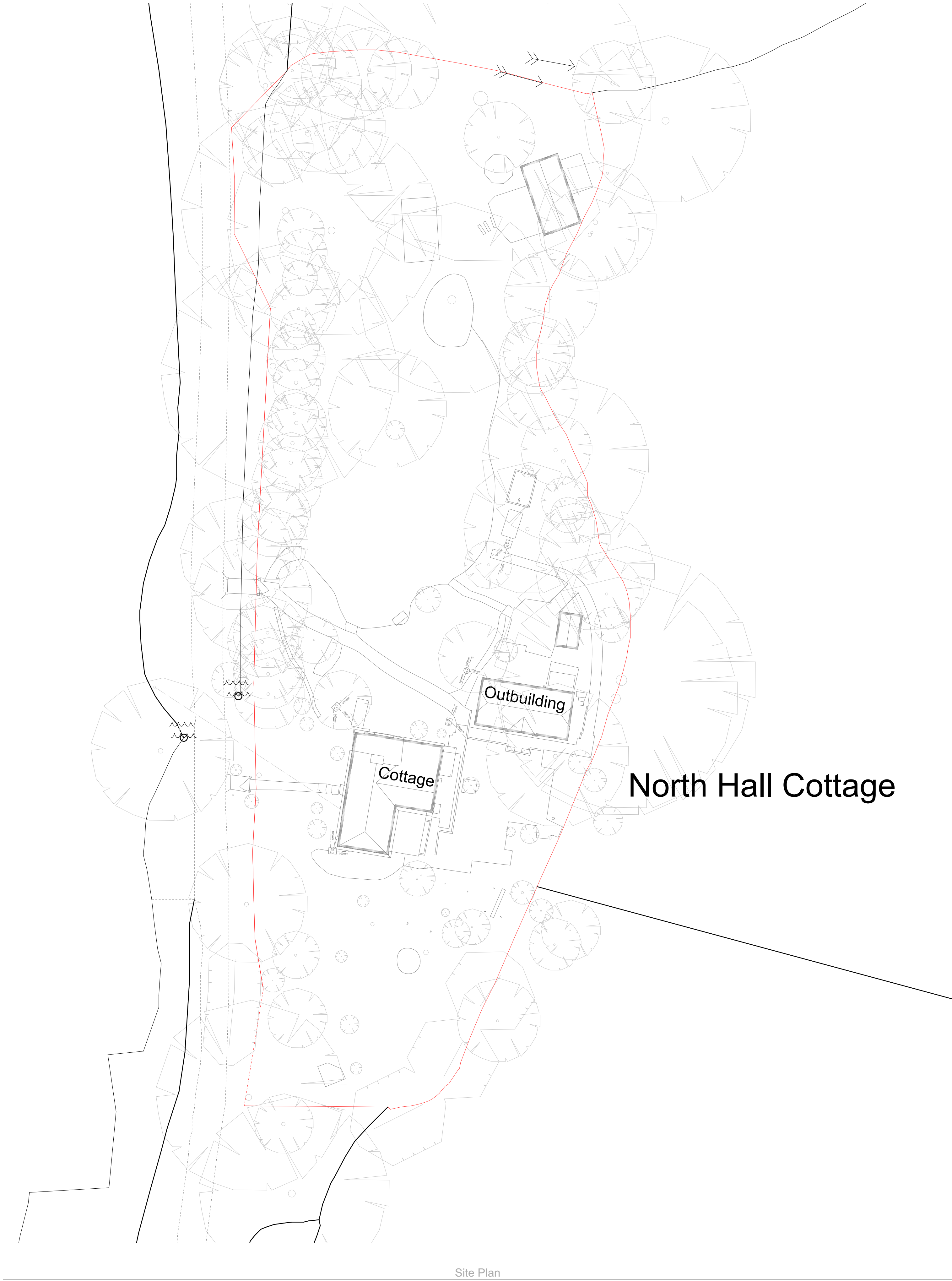
4.00 Drainage Strategy

- 4.01 Following the SuDS hierarchy, the provision of water butts is proposed as a water conservation measure.
- 4.02 Infiltration, if feasible, could be utilized as 5m away from any structure foundation can be achieved to place soakaways in line with relevant rules and regulations. However, based on the site geology, a conclusion that soil possesses negligible infiltration characteristics can be made.
- 4.03 The water from the impermeable roof is proposed to control discharge into the existing ditch with an attenuation tank sized for the 1 in a 100-year storm plus climate change allowance with the flow-controlled device orifice plate or similar.

5.00 Drainage Proposals

- 5.01 It is proposed to develop a residential extension to facilitate a kitchen dining and common bathroom and some internal alterations at the North Hall Cottage, Mallion's Lane, Staplefield, West Sussex, RH17 6AT.
- 5.02 For the consideration of surface water drainage, the contributing factor is circa 42 m² of the extension roof of the North Hall Cottage.
- 5.03 The impermeable roof areas are intended to be drained with conventional roof guttering and downpipes via 100mm and 150mm diameter pipes at appropriate gradients to ensure self-cleansing velocities, then directed into an attenuation tank and controlled discharge into the existing ditch via an orifice plate or similar approved at a flow rate of 0.70 l/s.
- 5.04 The attenuation and drainage system has been designed to accommodate a 1 in 100 years + 45% annual probability storm event, allowing for climate change in the critical storm. The tank at this site will be a crated system wrapped in an impermeable geotextile membrane.
- 5.05 Calculations also demonstrate that there is no risk of flooding of the drainage network for all durations of the 1 in 100-year storm event + 45% climate change (starting with 30 minutes and finishing with 24 hours storm). Calculations can be found in **Appendix D**.
- 5.06 The surface water drainage arrangement can be found in **Appendix E**.

Appendix A – Site Location Plan and Development Proposals

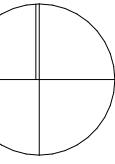


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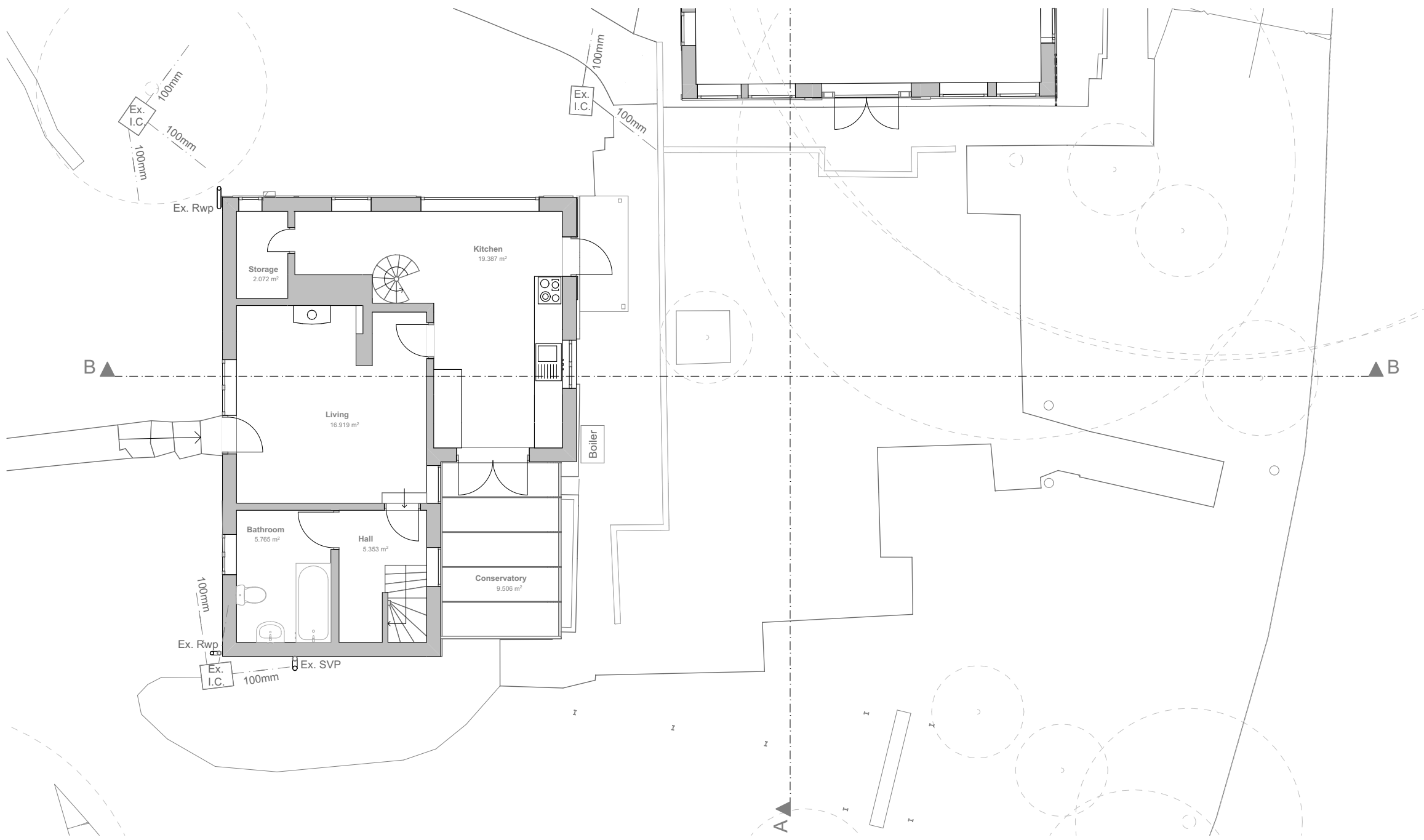
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Staplefield
RH17 6AT

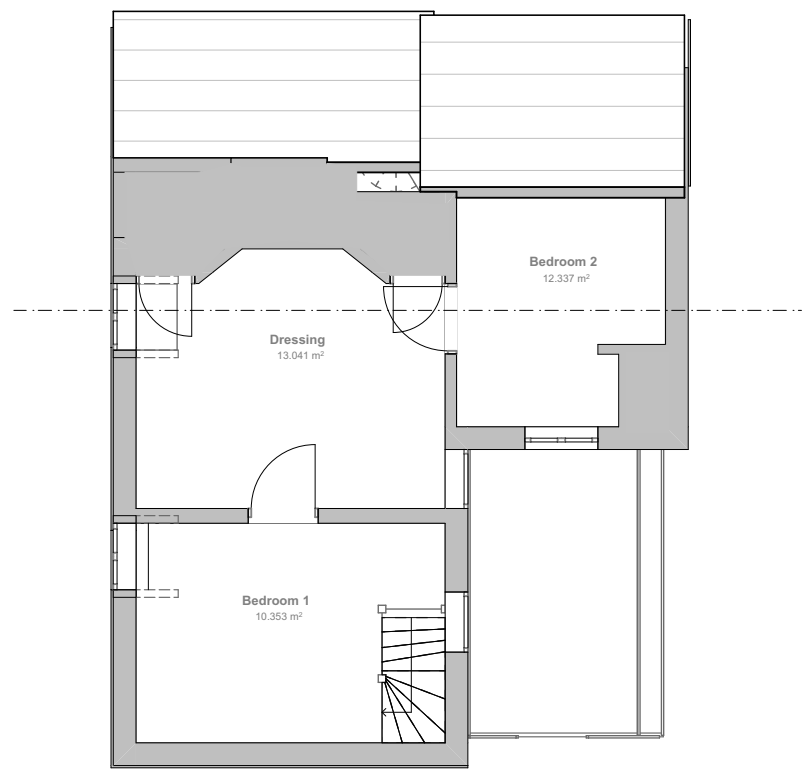
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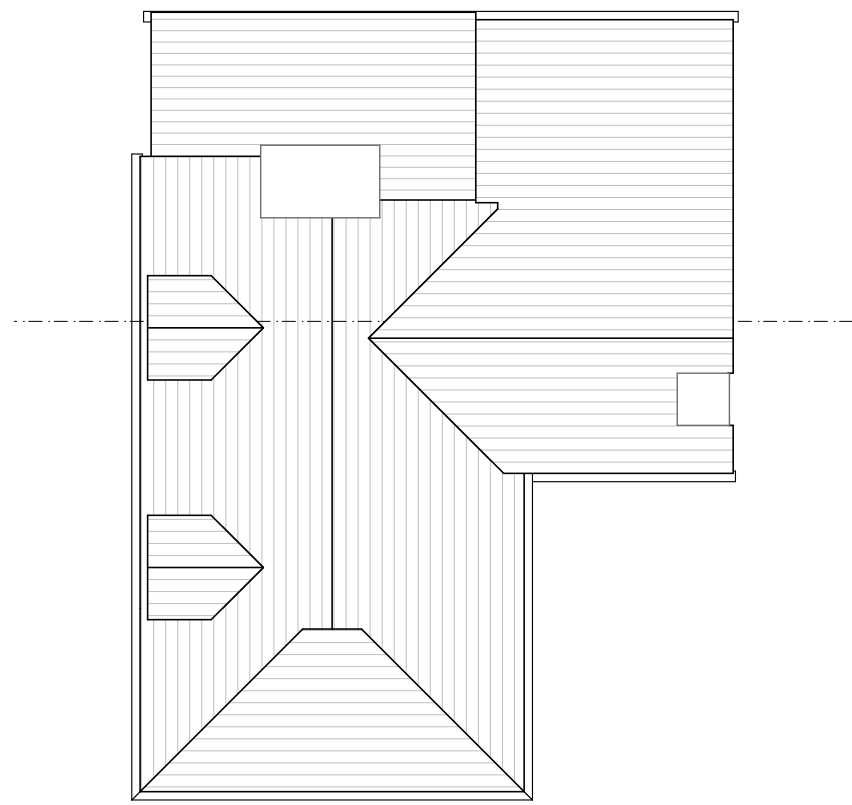
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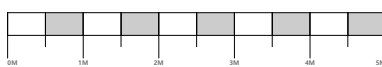
Ground Floor Plan



First Floor Plan

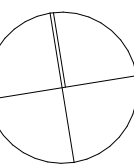


Roof Plan



Scale Bar

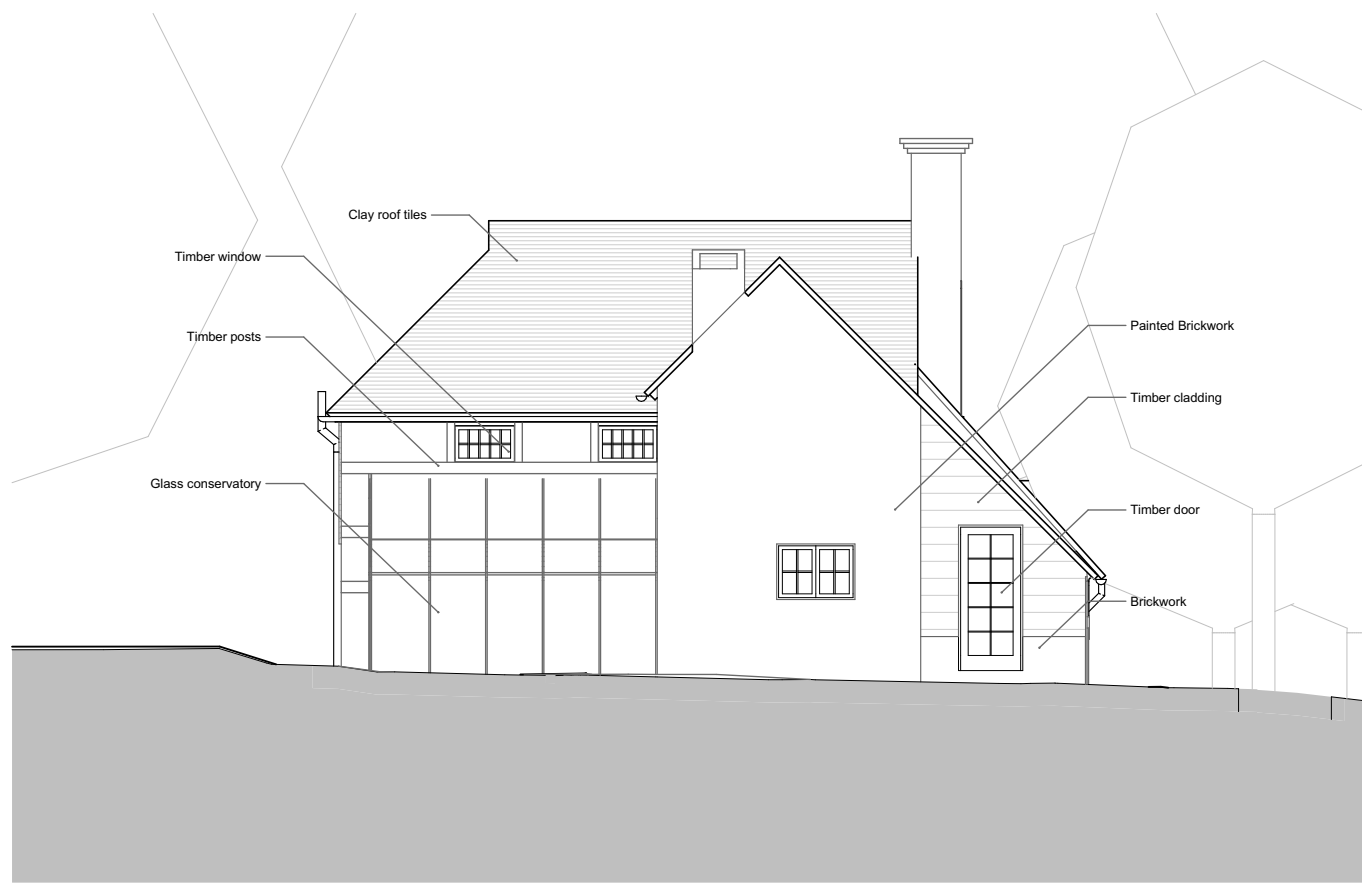
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Section BB



South Elevation



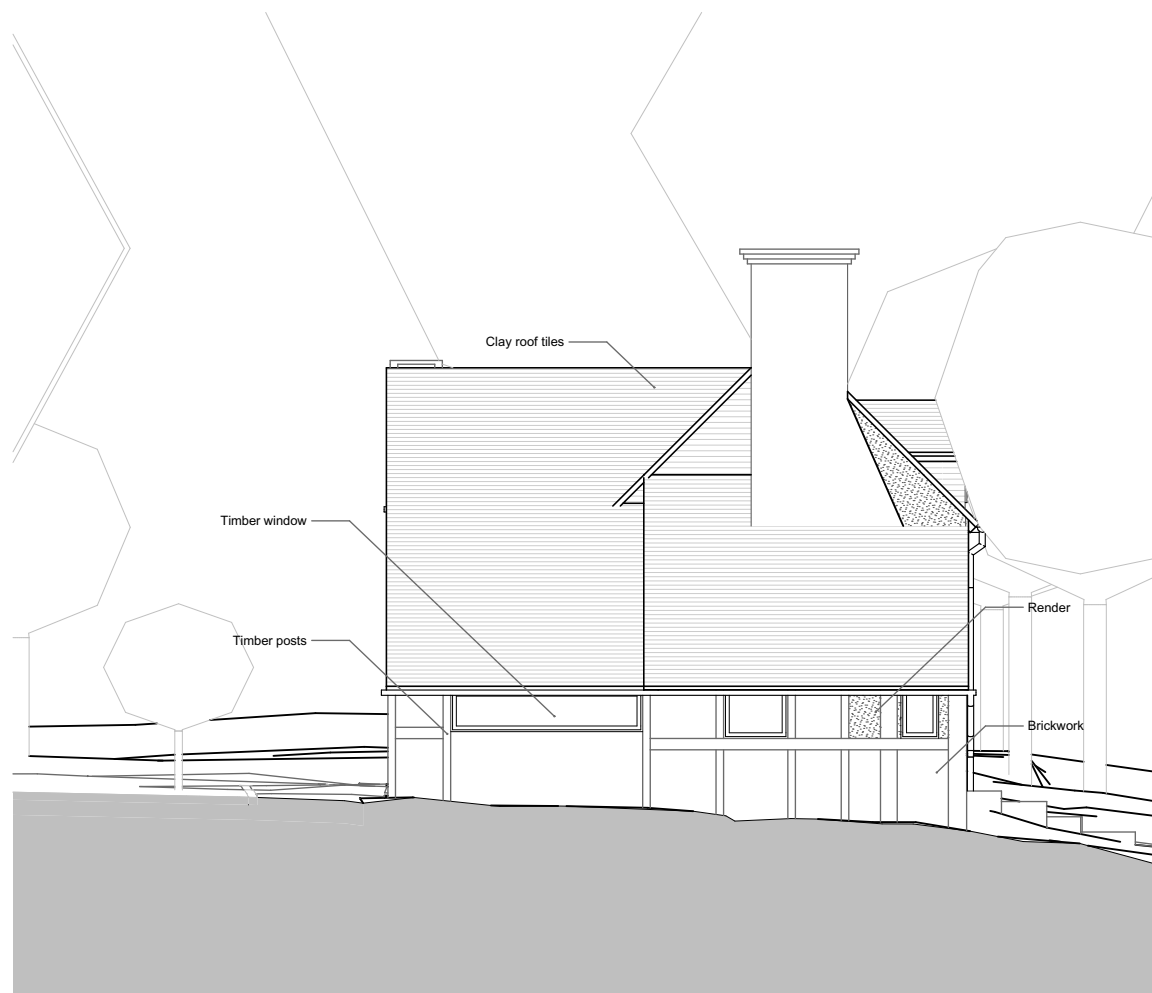
East Elevation



Section AA



West Elevation



North Elevation

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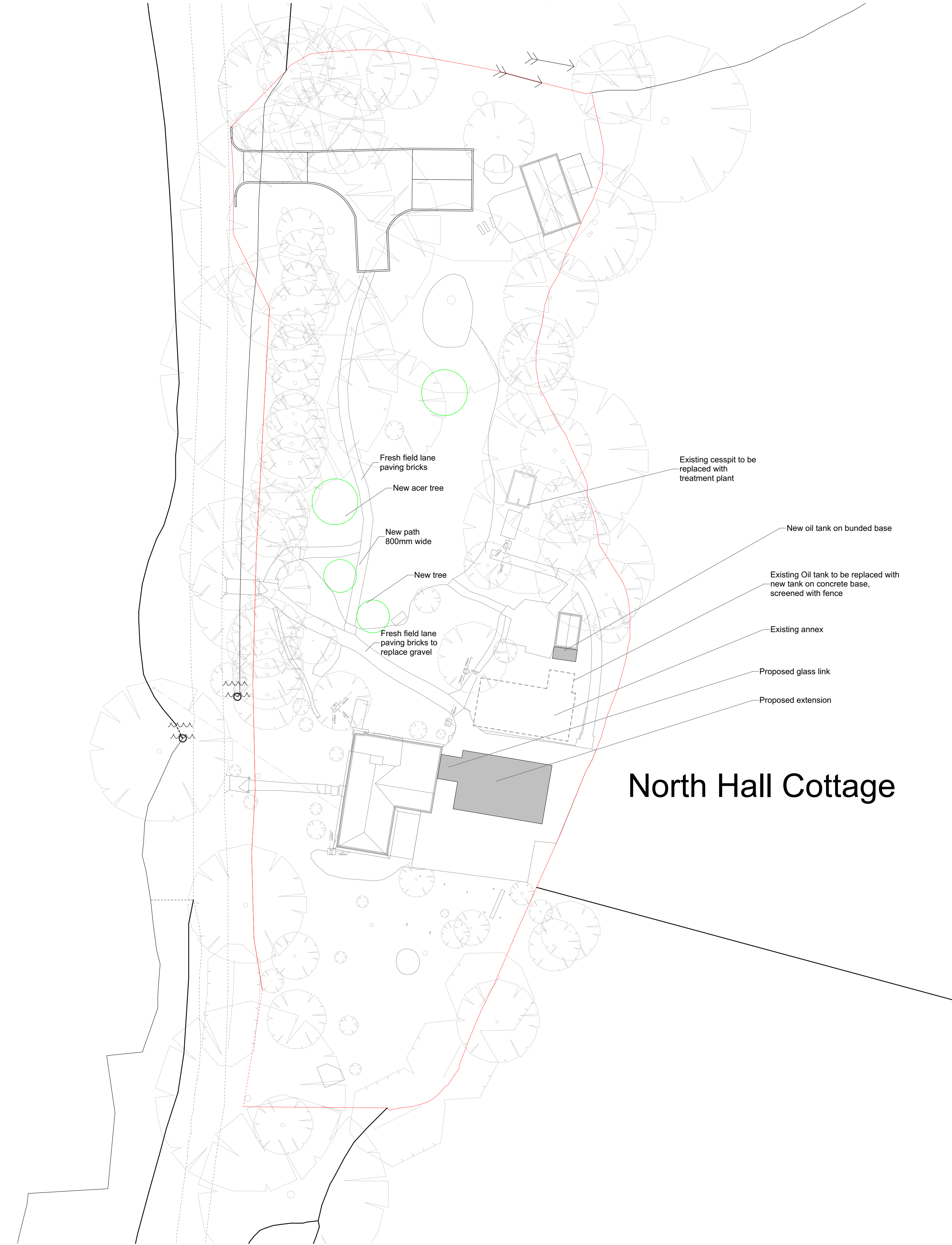
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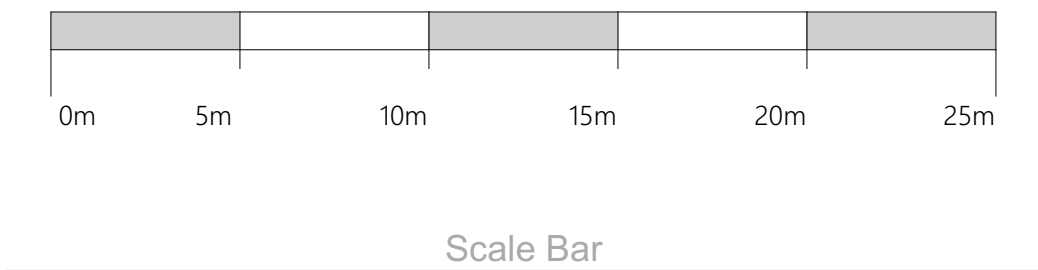
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Scale: 1:100 Date: Jun'23
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Site Plan



Scale Bar



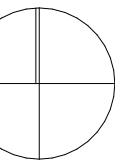
Location Plan

Notes

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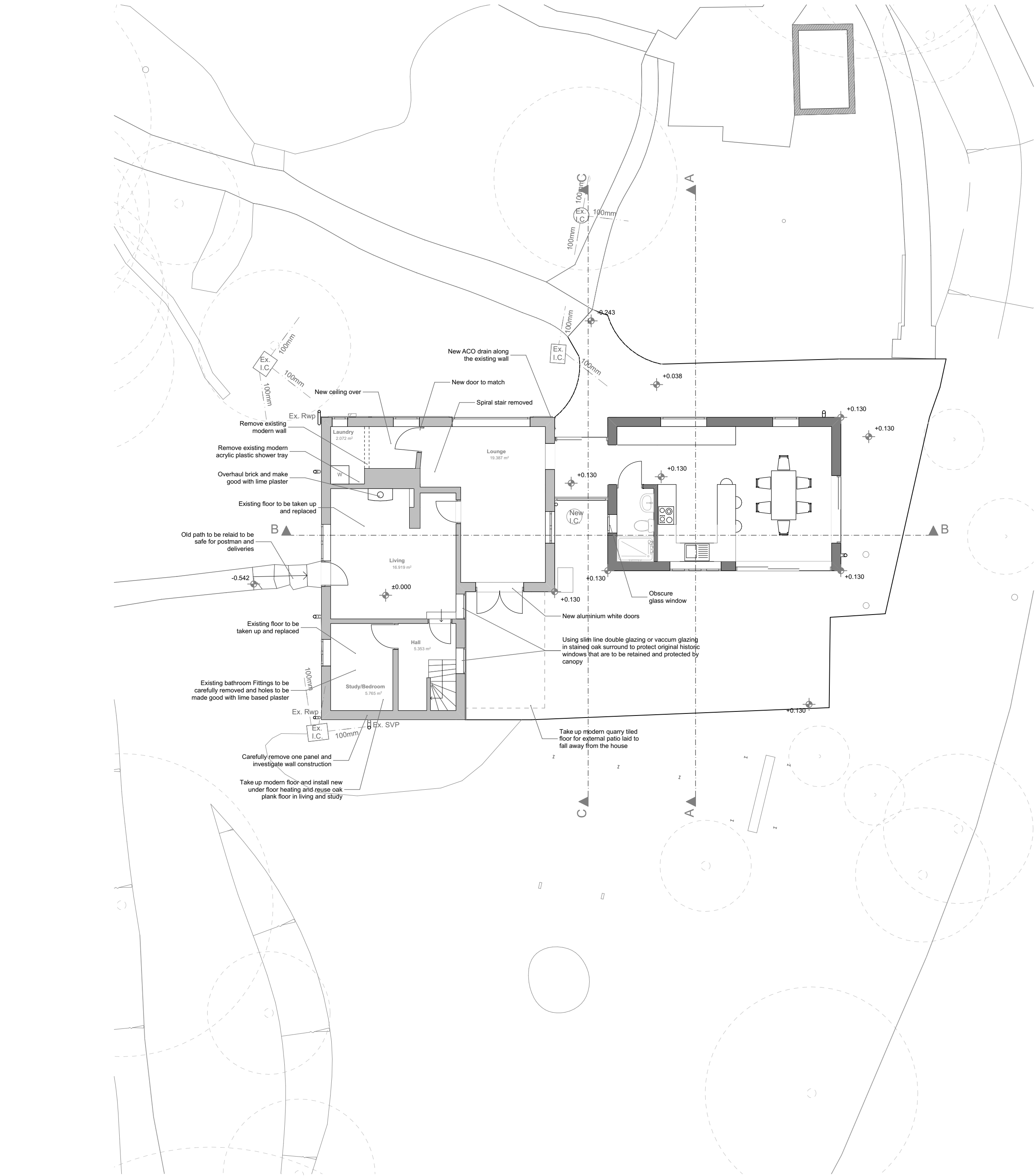
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Client: Chris and Sarah Mellish

Address: North Hall Cottage
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Staplefield
RH17 6AT

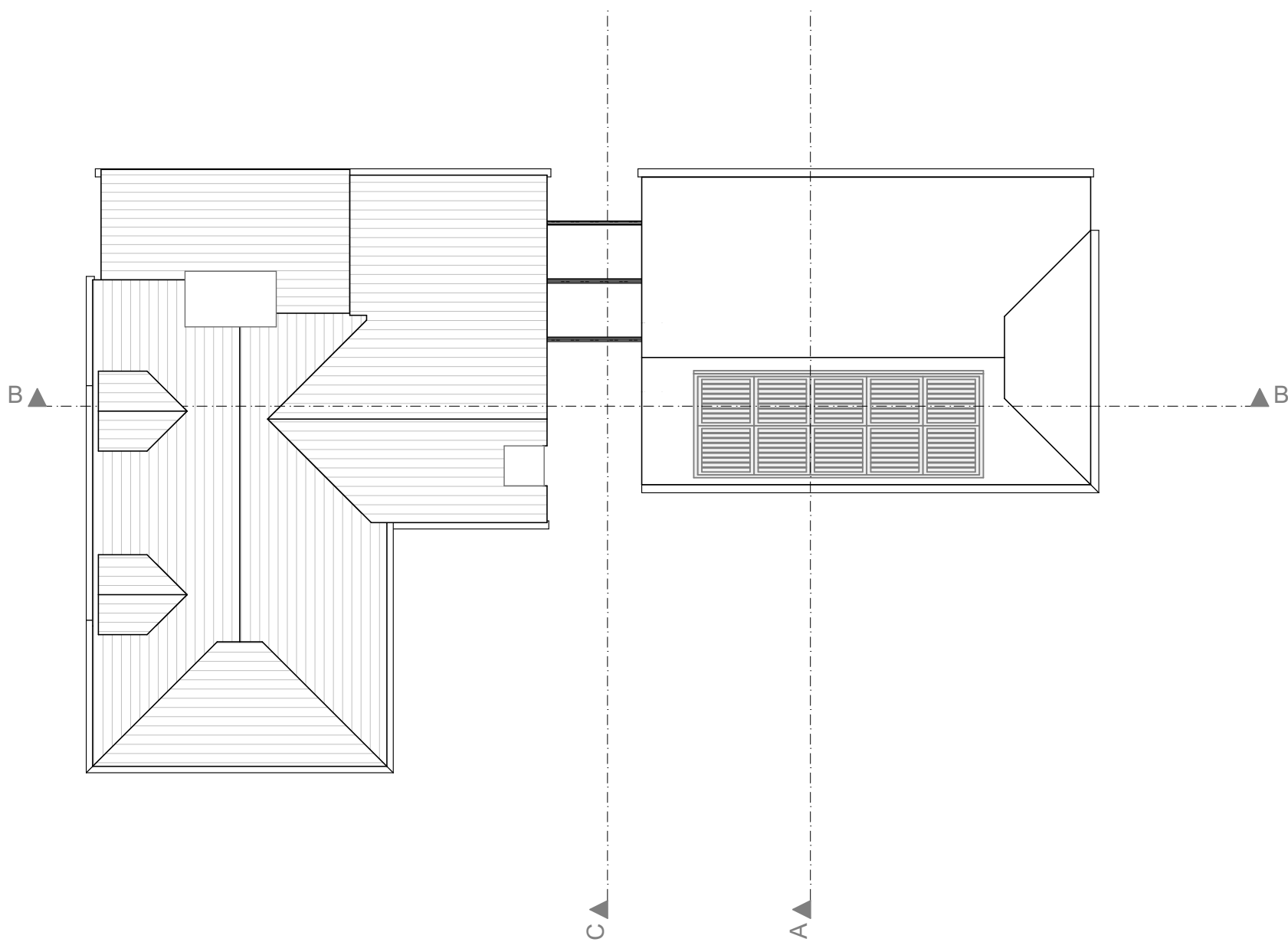
Description: Proposed site and location plan

Scale: 1:200, 1:1250 Date: Jun'23
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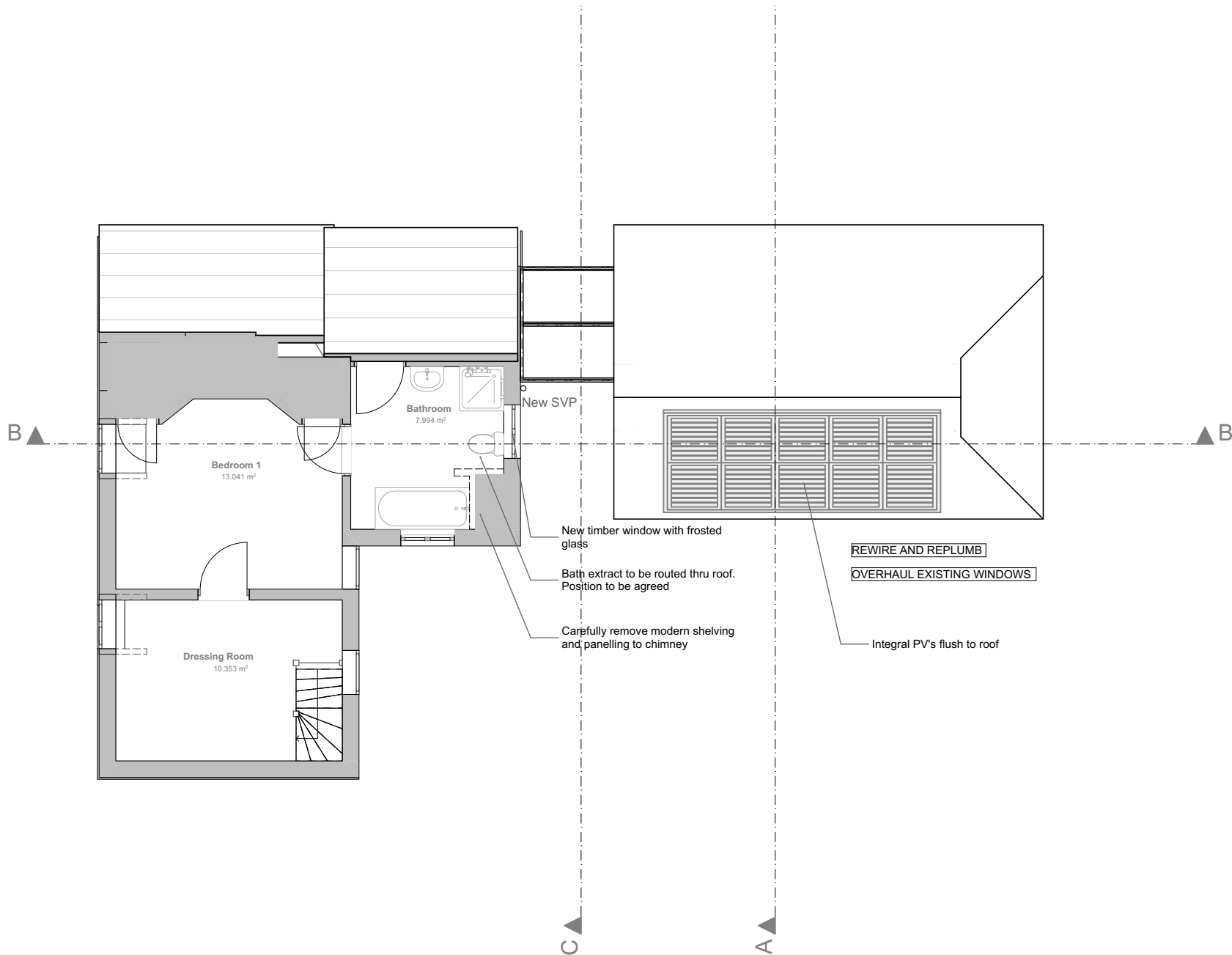
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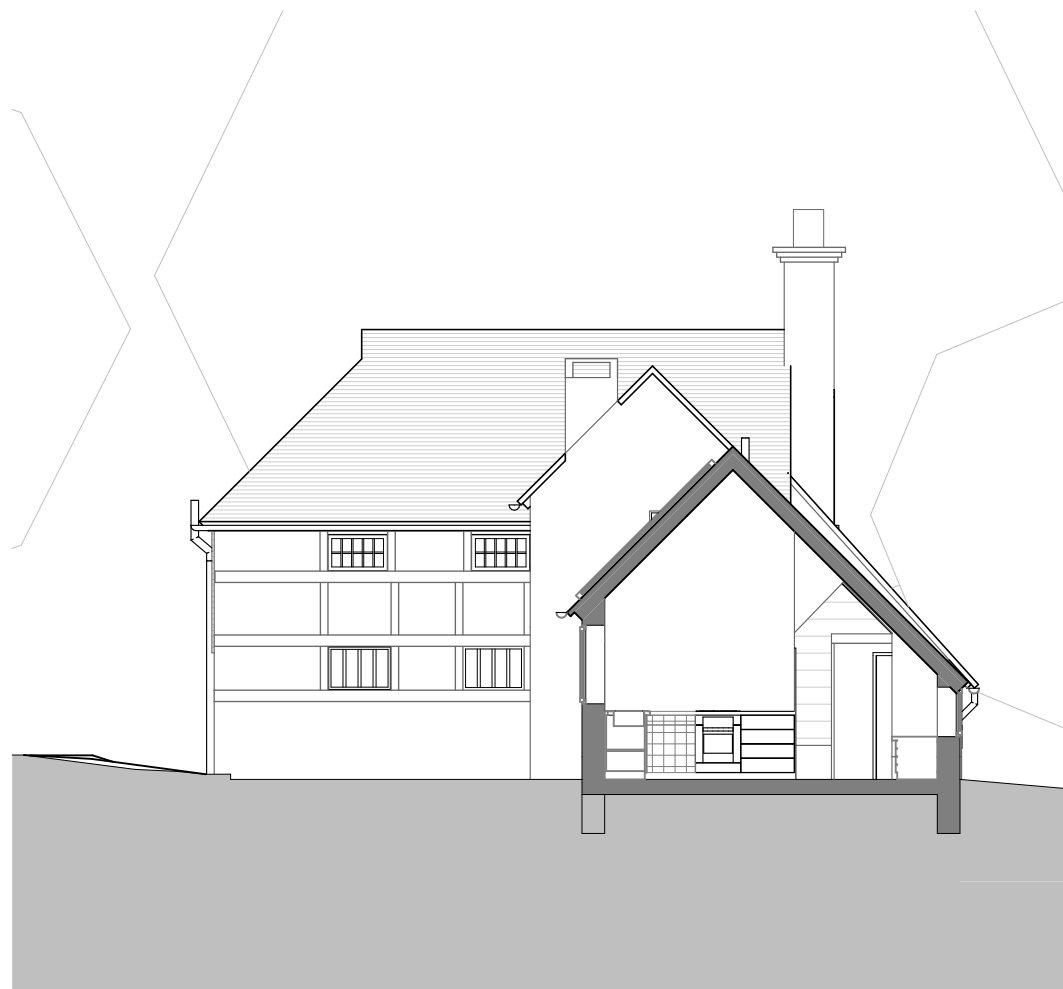
Ground Floor Plan



Roof Plan



First Floor Plan



Section AA



Section BB



Section CC

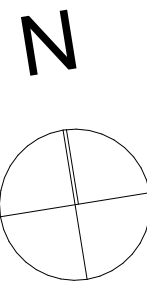
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Scale Bar



Notes:

All Existing black Upvc gutters and downpipes to be replaced with heritage black round downpipes and deep flow gutters

Green oak feather edge boarding to extension 200 wide

Existing wooden plank floor to annex to be used to replace living room and former bathroom floors

All existing casement windows to be repaired to match existing

Brickwork to extension plinth

Extension doors and windows to be PC Aluminium Black

01 - WIP	RevD	Transmitted By	Work in Progress	Jun'23	Layout Comment
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Description: Proposed cottage plans and sections

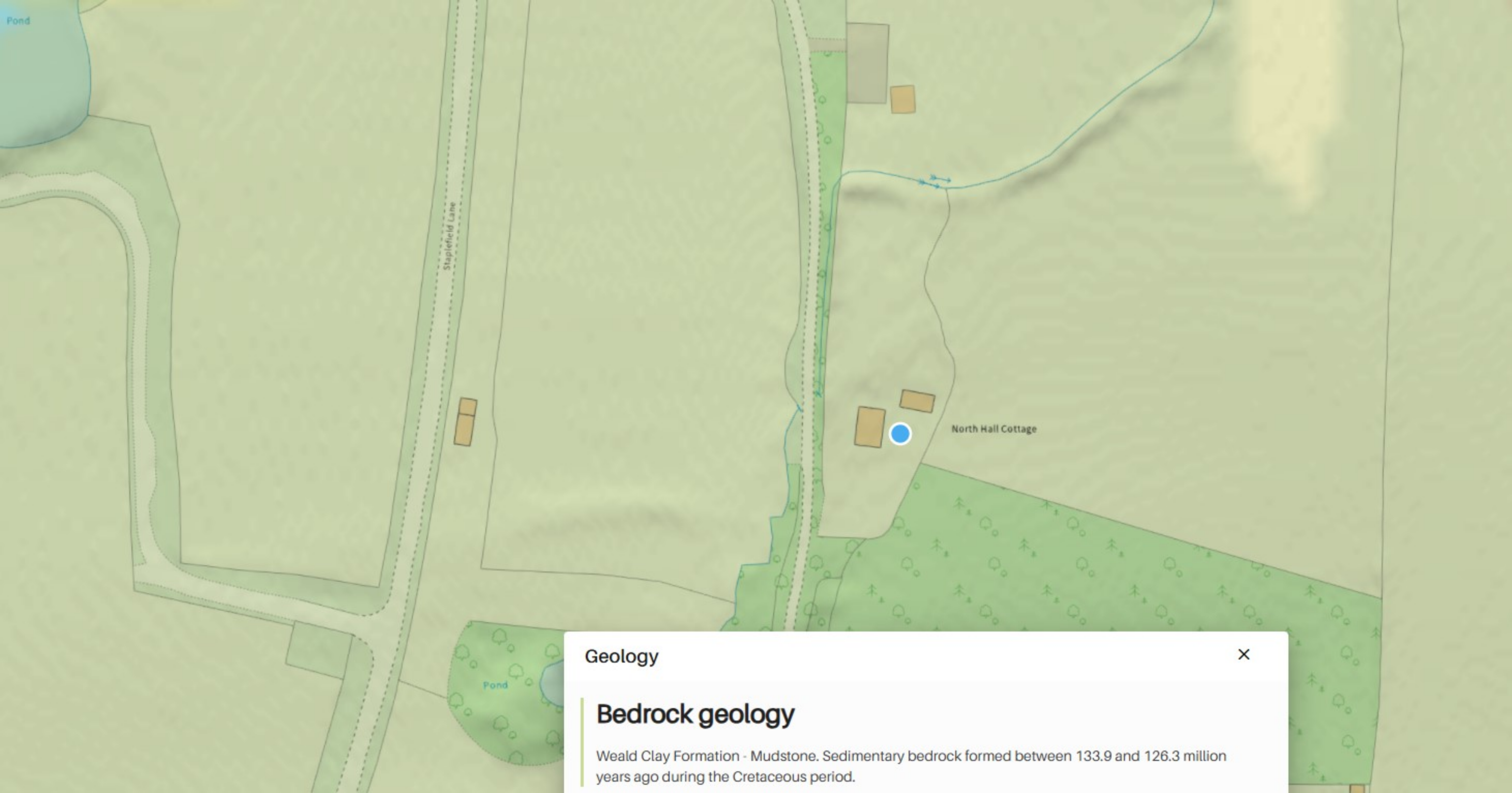
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Appendix B – Topographical Survey



Appendix C – Site Geology



Geology



Bedrock geology

Weald Clay Formation - Mudstone. Sedimentary bedrock formed between 133.9 and 126.3 million years ago during the Cretaceous period.

Appendix D – Preliminary Drainage Calculations (Critical Results)

Design Settings

Rainfall Methodology	FSR	Maximum Time of Concentration (mins)	30.00
Return Period (years)	1	Maximum Rainfall (mm/hr)	50.0
Additional Flow (%)	0	Minimum Velocity (m/s)	1.00
FSR Region	England and Wales	Connection Type	Level Soffits
M5-60 (mm)	26.300	Minimum Backdrop Height (m)	0.200
Ratio-R	0.400	Preferred Cover Depth (m)	1.200
CV	1.000	Include Intermediate Ground	✓
Time of Entry (mins)	5.00	Enforce best practice design rules	✓

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Depth (m)
SW.1	0.002	5.00	62.600	450	0.600
SW.2	0.002	5.00	62.600	450	0.700
RE.1	0.000	5.00	62.600	100	0.500
SW.03	0.000	5.00	62.150	600	0.600
AT	0.000	5.00	62.400	1200	0.800
Outfall	0.000	5.00	61.600		0.200

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	RE.1	SW.1	5.500	0.600	62.100	62.000	0.100	55.0	100	5.09	50.0
1.001	SW.1	SW.2	7.040	0.600	62.000	61.900	0.100	70.4	100	5.22	50.0
1.002	SW.2	AT	1.500	0.600	61.900	61.850	0.050	30.0	100	5.23	50.0
1.003	AT	SW.03	2.000	0.600	61.600	61.550	0.050	40.0	100	5.26	50.0
1.004	SW.03	Outfall	8.000	0.600	61.550	61.400	0.150	53.3	100	5.39	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	1.041	8.2	0.0	0.400	0.500	0.000	0.0	0	0.000
1.001	0.918	7.2	0.4	0.500	0.600	0.002	0.0	15	0.472
1.002	1.414	11.1	0.7	0.600	0.450	0.004	0.0	18	0.799
1.003	1.223	9.6	0.7	0.700	0.500	0.004	0.0	18	0.709
1.004	1.057	8.3	0.7	0.500	0.100	0.004	0.0	20	0.645

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
1.000	5.500	55.0	100	Circular	62.600	62.100	0.400	62.600	62.000	0.500
1.001	7.040	70.4	100	Circular	62.600	62.000	0.500	62.600	61.900	0.600
1.002	1.500	30.0	100	Circular	62.600	61.900	0.600	62.400	61.850	0.450
1.003	2.000	40.0	100	Circular	62.400	61.600	0.700	62.150	61.550	0.500
1.004	8.000	53.3	100	Circular	62.150	61.550	0.500	61.600	61.400	0.100

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
1.000	RE.1	100	Manhole	Adoptable	SW.1	450	Manhole	Adoptable
1.001	SW.1	450	Manhole	Adoptable	SW.2	450	Manhole	Adoptable
1.002	SW.2	450	Manhole	Adoptable	AT	1200	Manhole	Adoptable
1.003	AT	1200	Manhole	Adoptable	SW.03	600	Manhole	Adoptable
1.004	SW.03	600	Manhole	Adoptable	Outfall		Junction	

Manhole Schedule

Node	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)
SW.1	62.600	0.600	450	1	1.000	62.000	100
				0	1.001	62.000	100
SW.2	62.600	0.700	450	1	1.001	61.900	100
				0	1.002	61.900	100
RE.1	62.600	0.500	100	0	1.000	62.100	100
SW.03	62.150	0.600	600	1	1.003	61.550	100
				0	1.004	61.550	100
AT	62.400	0.800	1200	1	1.002	61.850	100
				0	1.003	61.600	100
Outfall	61.600	0.200		1	1.004	61.400	100

Simulation Settings

Rainfall Methodology	FSR	Analysis Speed	Normal
FSR Region	England and Wales	Skip Steady State	x
M5-60 (mm)	26.300	Drain Down Time (mins)	240
Ratio-R	0.400	Additional Storage (m³/ha)	20.0
Summer CV	1.000	Check Discharge Rate(s)	x
Winter CV	1.000	Check Discharge Volume	x

Storm Durations

30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720 | 960 | 1440

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
1	0	0	0
2	0	0	0
30	0	0	0
30	45	0	0
100	0	0	0
100	45	0	0

Node SW.03 Online Orifice Control

Flap Valve	x	Design Depth (m)	1.000	Discharge Coefficient	0.600
Replaces Downstream Link	✓	Design Flow (l/s)	0.7		
Invert Level (m)	61.550	Diameter (m)	0.018		

Node AT Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	61.600
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.96	Time to half empty (mins)	74

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	2.5	0.0	0.350	2.5	0.0	0.351	0.0	0.0

Results for 1 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	SW.1	19	62.016	0.016	0.4	0.0036	0.0000	OK
30 minute summer	SW.2	19	61.920	0.020	0.8	0.0043	0.0000	OK
30 minute summer	RE.1	1	62.100	0.000	0.0	0.0000	0.0000	OK
30 minute winter	SW.03	24	61.662	0.112	1.3	0.0318	0.0000	SURCHARGED
30 minute winter	AT	26	61.661	0.061	1.6	0.2159	0.0000	OK
30 minute summer	Outfall	1	61.400	0.000	0.2	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	SW.1	1.001	SW.2	0.4	0.420	0.055	0.0067	
30 minute summer	SW.2	1.002	AT	0.8	0.771	0.072	0.0016	
30 minute summer	RE.1	1.000	SW.1	0.0	0.000	0.000	0.0022	
30 minute winter	SW.03	Orifice	Outfall	0.2				0.6
30 minute winter	AT	1.003	SW.03	1.3	0.470	0.140	0.0128	

Results for 2 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	SW.1	19	62.018	0.018	0.5	0.0040	0.0000	OK
30 minute summer	SW.2	19	61.922	0.022	1.0	0.0048	0.0000	OK
30 minute summer	RE.1	1	62.100	0.000	0.0	0.0000	0.0000	OK
60 minute summer	SW.03	42	61.684	0.134	1.3	0.0379	0.0000	SURCHARGED
60 minute summer	AT	42	61.684	0.084	1.2	0.2961	0.0000	OK
30 minute summer	Outfall	1	61.400	0.000	0.2	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	SW.1	1.001	SW.2	0.5	0.445	0.069	0.0079	
30 minute summer	SW.2	1.002	AT	1.0	0.817	0.090	0.0018	
30 minute summer	RE.1	1.000	SW.1	0.0	0.000	0.000	0.0026	
60 minute summer	SW.03	Orifice	Outfall	0.2				0.8
60 minute summer	AT	1.003	SW.03	1.3	0.470	0.138	0.0148	

Results for 30 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	SW.1	18	62.025	0.025	1.0	0.0057	0.0000	OK
30 minute summer	SW.2	18	61.933	0.033	2.0	0.0071	0.0000	OK
30 minute summer	RE.1	1	62.100	0.000	0.0	0.0000	0.0000	OK
60 minute summer	SW.03	45	61.792	0.242	0.7	0.0684	0.0000	SURCHARGED
60 minute summer	AT	45	61.792	0.192	1.6	0.6775	0.0000	SURCHARGED
30 minute summer	Outfall	1	61.400	0.000	0.3	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	SW.1	1.001	SW.2	1.0	0.528	0.138	0.0134	
30 minute summer	SW.2	1.002	AT	2.0	0.970	0.180	0.0031	
30 minute summer	RE.1	1.000	SW.1	0.0	0.000	0.000	0.0042	
60 minute summer	SW.03	Orifice	Outfall	0.3				1.6
60 minute summer	AT	1.003	SW.03	0.7	0.470	0.071	0.0156	

Results for 30 year +45% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	SW.1	18	62.031	0.031	1.5	0.0070	0.0000	OK
30 minute summer	SW.2	18	61.942	0.042	3.0	0.0091	0.0000	OK
30 minute summer	RE.1	1	62.100	0.000	0.0	0.0000	0.0000	OK
60 minute winter	SW.03	50	61.915	0.365	1.0	0.1032	0.0000	FLOOD RISK
60 minute winter	AT	50	61.915	0.315	1.6	1.1111	0.0000	SURCHARGED
30 minute summer	Outfall	1	61.400	0.000	0.4	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	SW.1	1.001	SW.2	1.5	0.578	0.208	0.0183	
30 minute summer	SW.2	1.002	AT	3.0	1.068	0.270	0.0042	
30 minute summer	RE.1	1.000	SW.1	0.0	0.000	0.000	0.0057	
60 minute winter	SW.03	Orifice	Outfall	0.4				2.3
60 minute winter	AT	1.003	SW.03	1.0	0.470	0.105	0.0156	

Results for 100 year Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	SW.1	18	62.030	0.030	1.4	0.0068	0.0000	OK
30 minute summer	SW.2	18	61.940	0.040	2.8	0.0087	0.0000	OK
30 minute summer	RE.1	1	62.100	0.000	0.0	0.0000	0.0000	OK
60 minute winter	SW.03	49	61.869	0.319	1.3	0.0902	0.0000	FLOOD RISK
60 minute winter	AT	49	61.869	0.269	1.4	0.9493	0.0000	SURCHARGED
30 minute summer	Outfall	1	61.400	0.000	0.4	0.0000	0.0000	OK

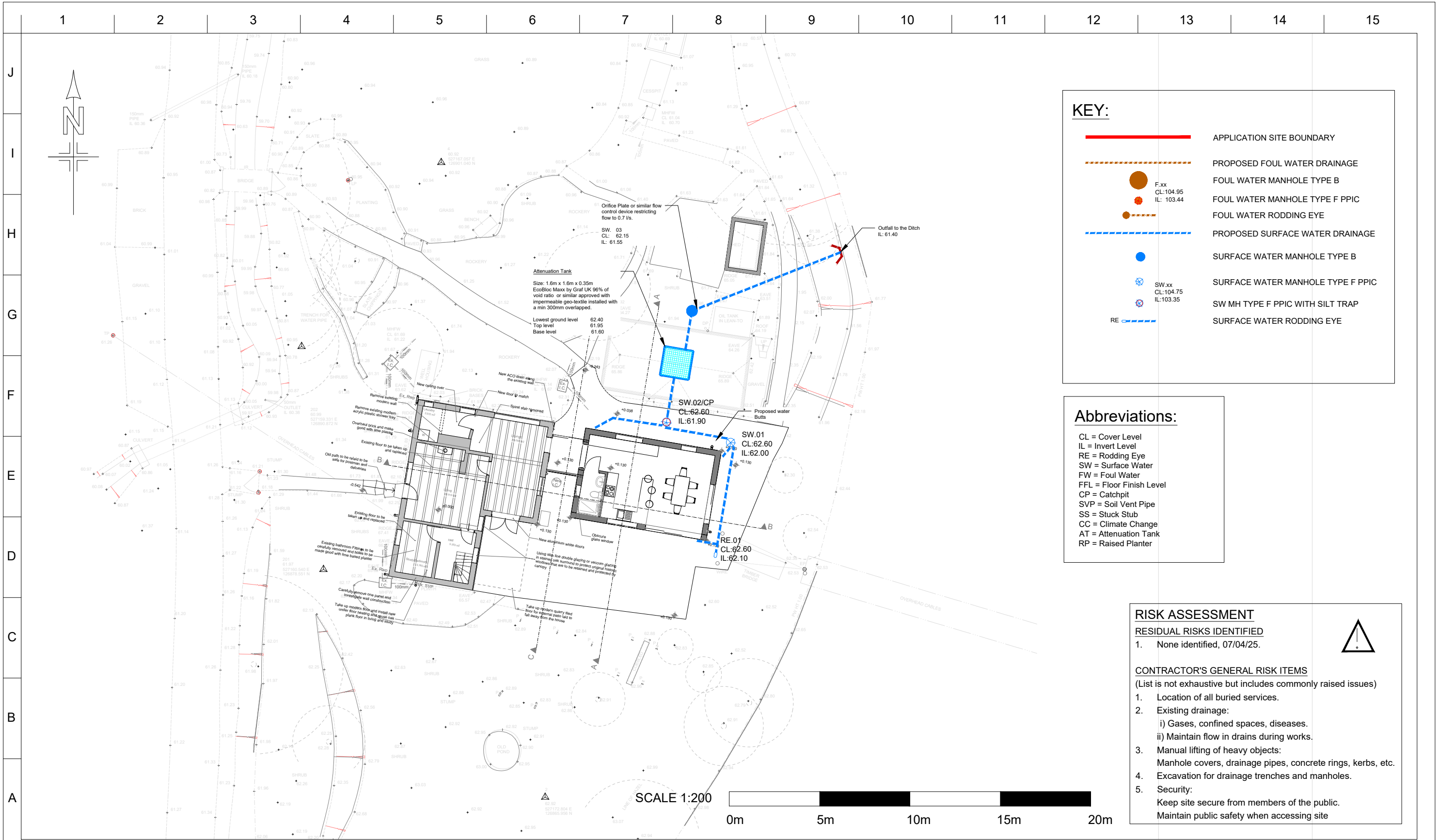
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	SW.1	1.001	SW.2	1.4	0.570	0.194	0.0173	
30 minute summer	SW.2	1.002	AT	2.8	1.051	0.252	0.0040	
30 minute summer	RE.1	1.000	SW.1	0.0	0.000	0.000	0.0054	
60 minute winter	SW.03	Orifice	Outfall	0.4				2.1
60 minute winter	AT	1.003	SW.03	1.3	0.470	0.134	0.0156	

Results for 100 year +45% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	SW.1	18	62.037	0.037	2.0	0.0083	0.0000	OK
60 minute summer	SW.2	47	62.036	0.136	2.8	0.0293	0.0000	SURCHARGED
30 minute summer	RE.1	1	62.100	0.000	0.0	0.0000	0.0000	OK
60 minute summer	SW.03	47	62.036	0.486	1.3	0.1374	0.0000	FLOOD RISK
60 minute summer	AT	47	62.036	0.436	2.8	1.3340	0.0000	SURCHARGED
30 minute summer	Outfall	1	61.400	0.000	0.5	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	SW.1	1.001	SW.2	2.0	0.612	0.277	0.0314	
60 minute summer	SW.2	1.002	AT	2.8	1.051	0.252	0.0117	
30 minute summer	RE.1	1.000	SW.1	0.0	0.000	0.000	0.0072	
60 minute summer	SW.03	Orifice	Outfall	0.5				3.1
60 minute summer	AT	1.003	SW.03	1.3	0.470	0.137	0.0156	

Appendix E – Surface Water Drainage Proposal



KEY:

APPLICATION SITE BOUNDARY

PROPOSED FOUL WATER DRAINAGE

FOUL WATER MANHOLE TYPE B

FOUL WATER MANHOLE TYPE F PPIC

FOUL WATER RODDING EYE

PROPOSED SURFACE WATER DRAINAGE

SURFACE WATER MANHOLE TYPE B

SURFACE WATER MANHOLE TYPE F PPIC

SW MH TYPE F PPIC WITH SILT TRAP

SURFACE WATER RODDING EYE

Abbreviations:

CL = Cover Level

IL = Invert Level

RE = Rodding Eye

SW = Surface Water

FW = Foul Water

FFL = Floor Finish Level

CP = Catchpit

SVP = Soil Vent Pipe

SS = Stuck Stub

CC = Climate Change

AT = Attenuation Tank

RP = Raised Planter

RISK ASSESSMENT

RESIDUAL RISKS IDENTIFIED

1. None identified, 07/04/25.

CONTRACTOR'S GENERAL RISK ITEMS

(List is not exhaustive but includes commonly raised issues)

1. Location of all buried services.

2. Existing drainage:

i) Gases, confined spaces, diseases.

ii) Maintain flow in drains during works.

3. Manual lifting of heavy objects:

Manhole covers, drainage pipes, concrete rings, kerbs, etc.

4. Excavation for drainage trenches and manholes.

5. Security:

Keep site secure from members of the public.

Maintain public safety when accessing site

- Notes:
1. This drawing to be read in conjunction with all other relevant Monson drawings and specifications, any discrepancies to be reported to Monson prior to any fabrication or work commencing on site.

2. Do not scale this drawing use figured dimensions only

3. All dimensions in millimetres, all levels in metres.

B	CL	09/04/2025	Attenuation tank update
A	CL	07/04/2025	Original Issue
Issue	By	Date	Amendments

MONSON

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Project:

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Mallion's Lane

Staplefield

RH17 6AT

Original Drawing:	Prepared	CL	Approved	MW
Origin:	Crowborough		Size	A3
Current Issue:	Approved	MW	Date	09/04/2025
Drawing Status:	PRELIMINARY			

Description:

Drainage Layout

Surface Water

Scale:	Dwg No:	Issue:
1:200	25127/01	B
1:200		