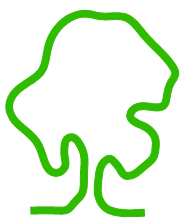


PHILLIP AND JULIE HARRIS

**LAND ADJOINING EVERGREEN, LONDON ROAD,
HASOCKS, WEST SUSSEX, BN6 9NA**

Flood Risk Assessment & Drainage Strategy

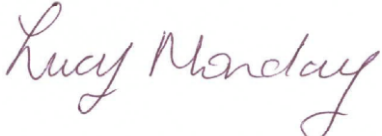
June 2026
(Revised August 2026)



eas ltd

Environmental Assessment Services Ltd

REPORT DATA SHEET

Requirement	Data
Report Ref	893/SDR/Evergreen/FRA
Date	June 2026
Client	Phillip and Julie Harris
Report type	Flood Risk Assessment & Drainage Strategy
Purpose	Planning
Revisions	Revised August 2026
Prepared by	Eur Ing Malcolm McKemey BSc (Hons), CEng, CEnv, MICE, MCIWEM, MIEnvSc  Signed
Approved by	Lucy Monday BSc (Hons), ACIEEM  Signed

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LAND ADJOINING EVERGREEN, LONDON ROAD,
HASOCKS, WEST SUSSEX, BN6 9NA

Flood Risk Assessment & Drainage Strategy

June 2026
(Revised August 2026)

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PHILLIP AND JULIE HARRIS

LAND ADJOINING EVERGREEN, LONDON ROAD,
HASOCKS, WEST SUSSEX, BN6 9NA

Flood Risk Assessment & Drainage Strategy

June 2026
(Revised August 2026)

1. BACKGROUND, EXISTING SITE AND PROPOSED DEVELOPMENT

- 1.1 The site is located to the north of an existing residential property known as Evergreen, to east of London Road (A273), which extends north of the village of Hassocks. The Ordnance Survey map reference for the centre of the site is TQ 30307 16722. The site elevation is approximately +34 m OD. See Appendix A; Figure 1 for the site location and Figure 2 for the proposed development plan.
- 1.2 The site (as existing) comprises the western half of a small field /paddock which has been used for grazing in the past.
- 1.3 It is proposed to construct three detached residential dwellings with surround garden areas and parking with access from London Road. See Appendix A; Figure 3 for the proposed development plan.

2. GEOLOGY & HYDROGEOLOGICAL CONSIDERATIONS

- 2.1 According to the British Geological Survey, the site lies on River Terrace Deposits (late Pleistocene) over Cretaceous Weald Clay. The river terrace deposits are described as “sand & gravel”, although the substrate in this area would suggest a high proportion of clay and silt in the deposits.
- 2.2 The River Terrace Deposits are a Secondary A Aquifer and the under lying Weald Clay is classified as Unproductive Strata. The site is not within a groundwater Source Protection Zone (SPZ), has low groundwater vulnerability and the site is only moderately hydrogeologically sensitive.

3. FLOOD RISK ASSESSMENT

- 3.1 According to the Environment Agency Flood Risk mapping, the red line boundary of the site lies entirely in Flood Zone 1 (at low risk of flooding from fluvial sources). This is guesstimated to remain the same in the 2070 – 2125 scenario. See Appendix B.
- 3.2 The site is not shown to be at risk of surface water (pluvial) flooding in the present or the 2070 – 2125 scenario. See Appendix B.

- 3.3 Mid-Sussex District Council has stated that the site is at increased risk of surface water flooding in the 2070 – 2125 scenario, however this does not appear to be the case according to the latest flood risk mapping from the Environment Agency. See Appendix B.
- 3.4 Groundwater flooding or flooding from reservoirs is deemed unlikely at this location.
- 3.5 The nearest Flood Zone 2 is some 12 m to the east of the site and the nearest Flood Zone 3 is some 25 m to the east, both associated with a significant watercourse (a tributary of the River Adur) some 100 m to the east of the site.

4. ALTERNATIVE SOURCES OF FLOODING

- 4.1 The remaining feasible source of flooding would be flooding from sewers. There is a public foul sewer passing the site in London Road. The sewer is 225 mm diameter and presently starts at chamber No. 2702, a short distance to the north of the site. The second chamber (No. 2701) is opposite the site.
- 4.2 There does not appear to be any record of sewer flooding affecting the site.

5. HISTORY OF FLOODING

- 5.1 No record of flooding affecting the site has been found. However, as the site is undeveloped, it is possible that flooding may have occurred and been unreported.
- 5.2 Mid Sussex District Council has advised that they hold records of the site, area around the site and adjacent highway having flooded in the past. No event data, historic flood mapping or aerial photography have been provided or found to confirm this.

6. PLANNING WITH RESPECT TO FLOODING

- 6.1 The Sequential Test is designed to encourage “more vulnerable” development (e.g. residential developments) to locate in areas of lower flood risk. In this case, the location is within Flood Zone 1 (low fluvial flood risk), not at risk of surface water flooding.
- 6.2 Mid Sussex District Council have stated that the site is at higher risk of surface water flooding in the 2070 – 2125 scenario. Assuming that were true, it could be considered that the site was in Flood Zone 2, still appropriate for “more vulnerable” development under current guidelines.
- 6.3 The Sequential Test would appear to be satisfied and there is no reason to apply the Exception Test.

7. RISK TO PROPERTY

- 7.1 As long as the finished floor level is at least 300 mm above the surrounding ground level or 600 mm above the nearest design flood level, whichever is the higher, there should be no significant flood related risk to property.

8. RISK TO USERS OF THE PROPOSED DEVELOPMENT

- 8.1 The proposed residents will be unrestricted in their use of the site as it will remain within Flood Zone 1 (or equivalent Flood Zone 2 in the 2070 – 2125 scenario, according to Mid-Sussex District Council).
- 8.2 According to the surface water flood mapping, the adjacent London Road highway will have standing water in the depth range 0 – 150 mm and some small areas in the depth range 150 - 300 mm. In the 2070 – 2125 scenario, the proportion of the highway in the depth range of 150 – 300 mm is shown increased. Table 8.1 below summarises the risks associated with the likely depths of water.

TABLE 8.1

Water Depth	Wading	Vehicle
0 – 150 mm	Hazardous to some	Hazardous to normal cars
150 – 300 mm	Hazardous to most	Hazardous to most vehicles

- 8.3 The hazard will increase with flow velocity. According to the Ordnance Survey, the highway has a slight fall towards the north for 600 m, rising thereafter towards Burgess Hill. The road is moderately flat for 600 m to the south. The principal flow direction may then be across rather than along the road. No flow velocity data is available for this site.
- 8.4 It is apparent that the future residents should wait for the pluvial flood waters to fall before venturing out onto the road. How quickly the flood level will fall will depend on the highway drainage and whether/for how long the heavy rain has stopped.
- 8.5 The flood maps show the footpath/cycleway from the site towards Burgess Hill to be outside the surface water flooding as far as New Close Farmhouse, when some areas of 0 – 150 mm are shown, beyond which it remains dry north into Burgess Hill, including for the 2070 – 2125 scenario. In theory, this would represent a safe route in times of pluvial flooding. However, a short part of the route, where it crosses above a culvert to the south of New Close Farmhouse, is shown to be in Flood Zones 2 and 3. However, this may be due to the flood model assuming no height of land above the culvert.
- 8.6 Ambulances, fire engines and police SUVs would be anticipated to have enough ground clearance to safely pass along London Road to reach the site at a time of pluvial flooding.

9. LIKELY IMPACT ON FLOOD RISK ELSEWHERE

- 9.1 The proposed development will increase the impermeable area compared with the existing greenfield condition. This may increase peak flows from the site and possibly increase flood risk downstream from the site.
- 9.2 This risk may be avoided or mitigated by the use of sustainable drainage (SuDS) as part of the development, where offsite flows will not exceed the equivalent greenfield flow.

10. SURFACE WATER DRAINAGE STRATEGY

- 10.1 The preferred SuDS drainage method will be infiltration (drainage to soakaway) where no surface water will leave the site. This requires the underlying geology to be reasonably permeable. In this case, it is possible (although unlikely) that the river terrace material may be suitable, but the underlying Weald Clay is typically too impermeable for infiltration drainage. It will be necessary to carry out infiltration testing at the site to eliminate this option.
- 10.2 The alternative to drainage to soakaway will be attenuation storage with maximum outflow limited to a maximum of greenfield flow. The controlled flow may be discharged to the watercourse to the east of the site. There is adequate room within the site to provide attenuation storage in the form of a pond, swale or underground void formers.
- 10.3 Table 10.1 below gives the greenfield flow rates for the site (from UK SuDS).

TABLE 10.1

Return Period	Flow (l/s)
1 in 1 year	0.4
1 in 2 year	0.5
1 in 10 year	0.8
1 in 30 year	1.2
1 in 100 year	1.6
1 in 200 year	1.9

This suggests that the surface water flow leaving the site should not exceed 0.4 l/s. However, manufacturers advise that the lowest reliable flow achievable with a vortex valve is 2 l/s and an orifice plate with a diameter of less than 50 mm is at severe risk of blockage. The outline design is based on a maximum flow of 2 l/s.

- 10.4 A suggested drainage arrangement is shown in Figure 3. Chamber details are summarised in Table 10.2 below:

TABLE 10.2
SURFACE WATER DRAINAGE

Ref.	Dia (mm)	Cover level (mAOD)	Invert level (mAOD)	Pipe dia in (mm)	Pipe dia out (mm)
SW1	450	35.00	34.50	-	100
SW2	450	35.00	34.30	100	150
SW3	450	35.00	34.25	150	100
SW4	600	35.00	34.05	150	150
SW5	600	35.00	34.50	-	100
SW6	900	35.00	34.00	100 + 150	225
Atten	16200	34.70	33.20	225	100
SW7	1200	34.00	33.10	100	100

- 10.5 The design is based on the 30 year return period storm plus a 35% allowance for climate change and checked for surcharge against the 100 year return period storm plus a 45% allowance for climate change. With a free running outlet there would be no surcharge and there would be some surcharge in the lower chambers, not exceeding any warning levels, for a flooded outlet.

11. FOUL SEWERAGE STRATEGY

- 11.1 Foul sewage should be discharged to the public foul sewer, where there is one within practicable reach. In this case there is a public foul sewer in London Road and chamber No. 2701 (Cover level +34.86 m OD, Invert level +33.64 m OD) is adjacent to the site.
- 11.2 A proposed foul drainage arrangement is shown in Figure 3. Chamber details are summarised in Table 11.1 below:

TABLE 11.1
FOUL SEWERAGE

Ref.	Dia (mm)	Cover level (mAOD)	Invert level (mAOD)	Pipe dia in (mm)	Pipe dia out (mm)
F1	450	35.00	34.50	-	100
F2	450	35.00	34.32	100	100
F3	600	35.00	34.07	100	100
2701	1200	34.86	33.64	100 + 225	225

- 11.3 It will be necessary to apply to Southern Water for permission to connect to the foul sewer. Southern Water has already indicated that there is adequate capacity in the sewer.

12. CONCLUSIONS

- 12.1 The site is presently undeveloped. According to the British Geological Survey, the site lies on River Terrace Deposits (sand and gravel) over Weald Clay (mudstone).
- 12.2 The site lies entirely within Flood Zone 1 (at low risk of flooding from fluvial or tidal flooding). Climate change is not predicted to change this status. There are areas of Flood Zone 2 and Flood Zone 3 relatively close to the site associated with the significant watercourse to the east of the site.
- 12.3 The Environment Agency flood mapping shows the site to be at very low risk of surface water (pluvial) flooding. Groundwater and reservoir flooding are considered unlikely at this location. Climate change is not predicted to change this status.
- 12.4 Some puddling of rainwater is shown on the adjacent highway and some of the depths indicated could be hazardous for wading and standard cars. The cycleway and footpath adjacent to the site is shown unaffected by surface water flooding from the site all the way to Burgess Hill, apart from a small section over a culvert to the south of New Close Farmhouse (which may be a false indication from the model).
- 12.5 The Sequential Test is designed to encourage “more vulnerable” development (e.g. residential developments) to locate in areas of lower flood risk. The site is within Flood Zone 1 and is not at risk of surface water flooding. The Sequential Test would appear to be satisfied. There is no reason to apply the Exception Test.
- 12.6 Sustainable drainage (SuDS) will be required to minimise the risk of increasing the impact of flooding downstream from the site. Draining surface water to soakaway would be the preferred option. This will require infiltration testing at the site to see whether this would be practicable. An outline design has been prepared based on attenuation storage with a controlled flow outlet.
- 12.7 Foul sewage should be discharged to the public foul sewer in London Road. It will be necessary to apply to Southern Water for permission to connect.



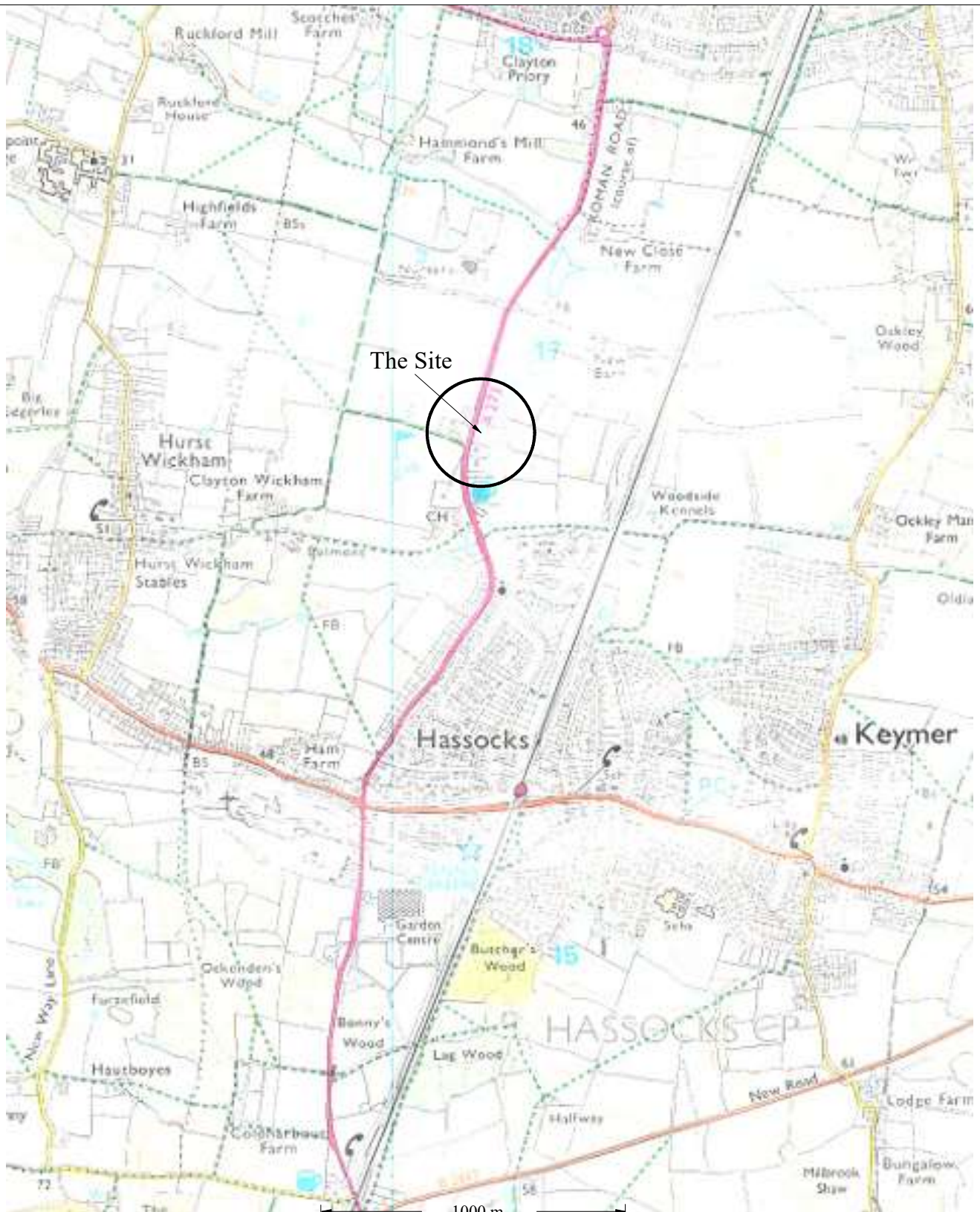
APPENDIX A

FIGURES

Figure 1: Site Location

Figure 2: Proposed Development

Figure 3: Suggested Drainage Arrangement



Reproduced from the Ordnance Survey 1:25000 scale map with permission. Licence No. 100005508

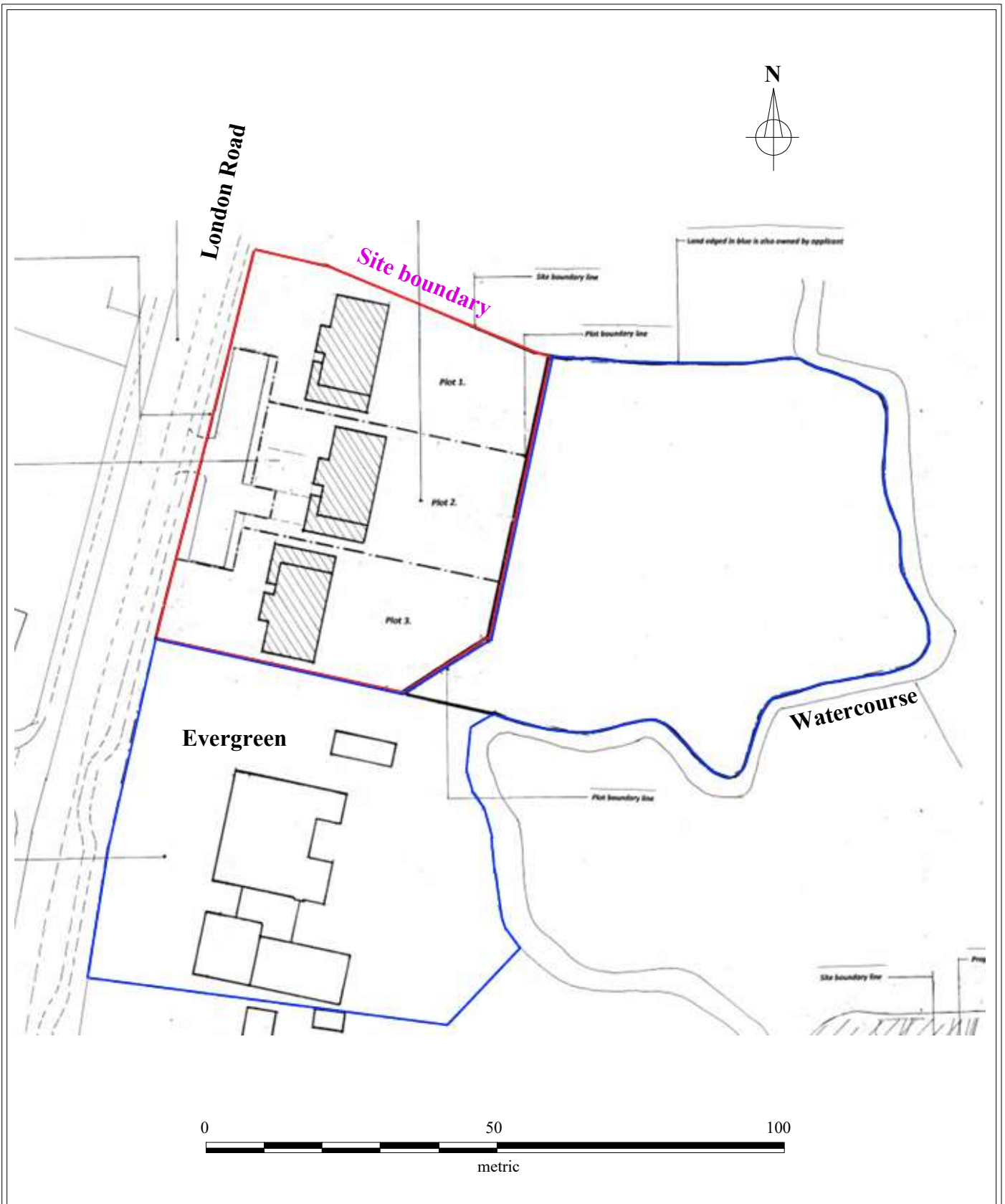
Scale as shown



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Figure 1: Site Location

June 2026

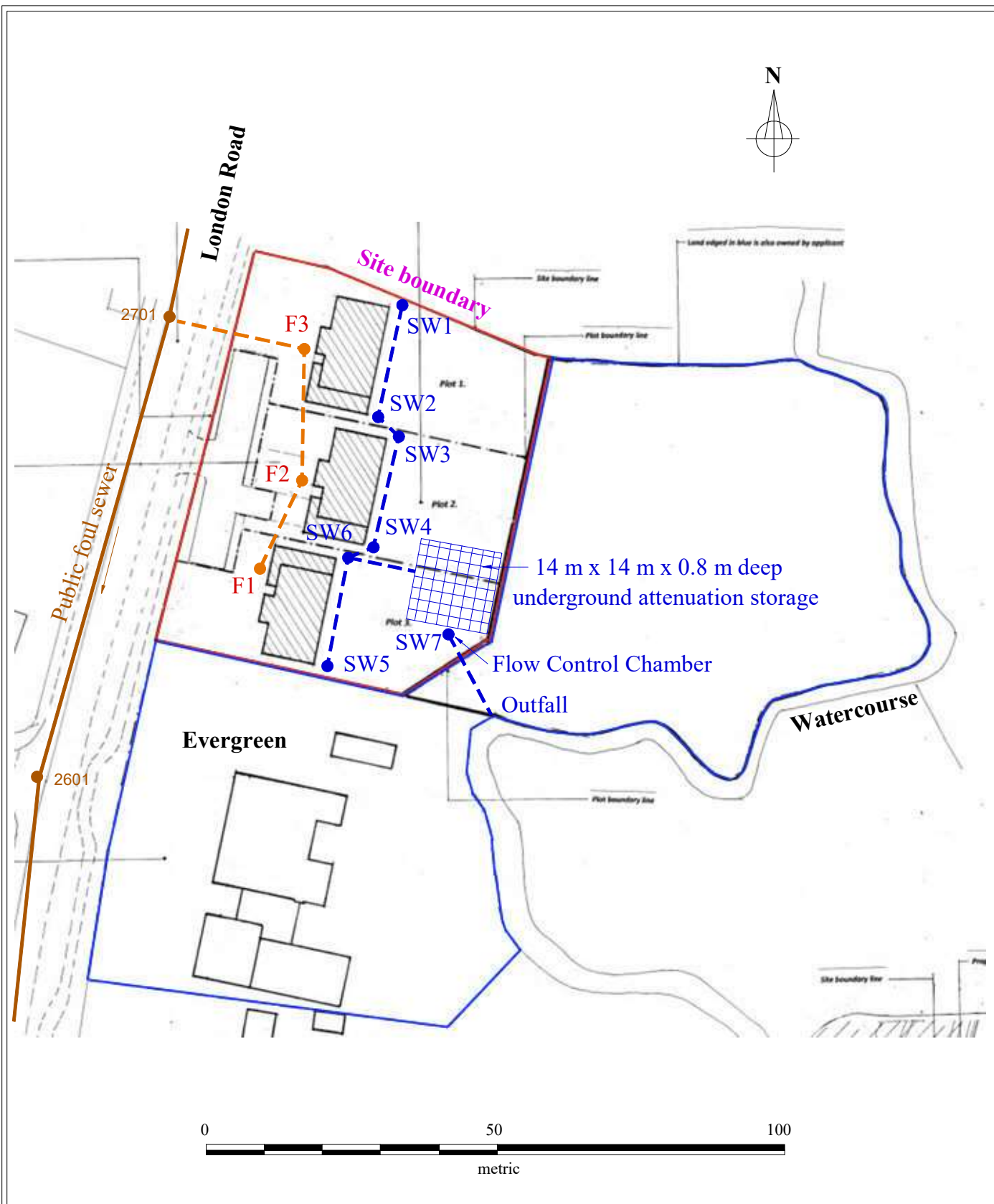


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Figure 2: The site and Proposed Development

Scale as shown

June 2026



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 WEST SUSSEX, BN6 9NA

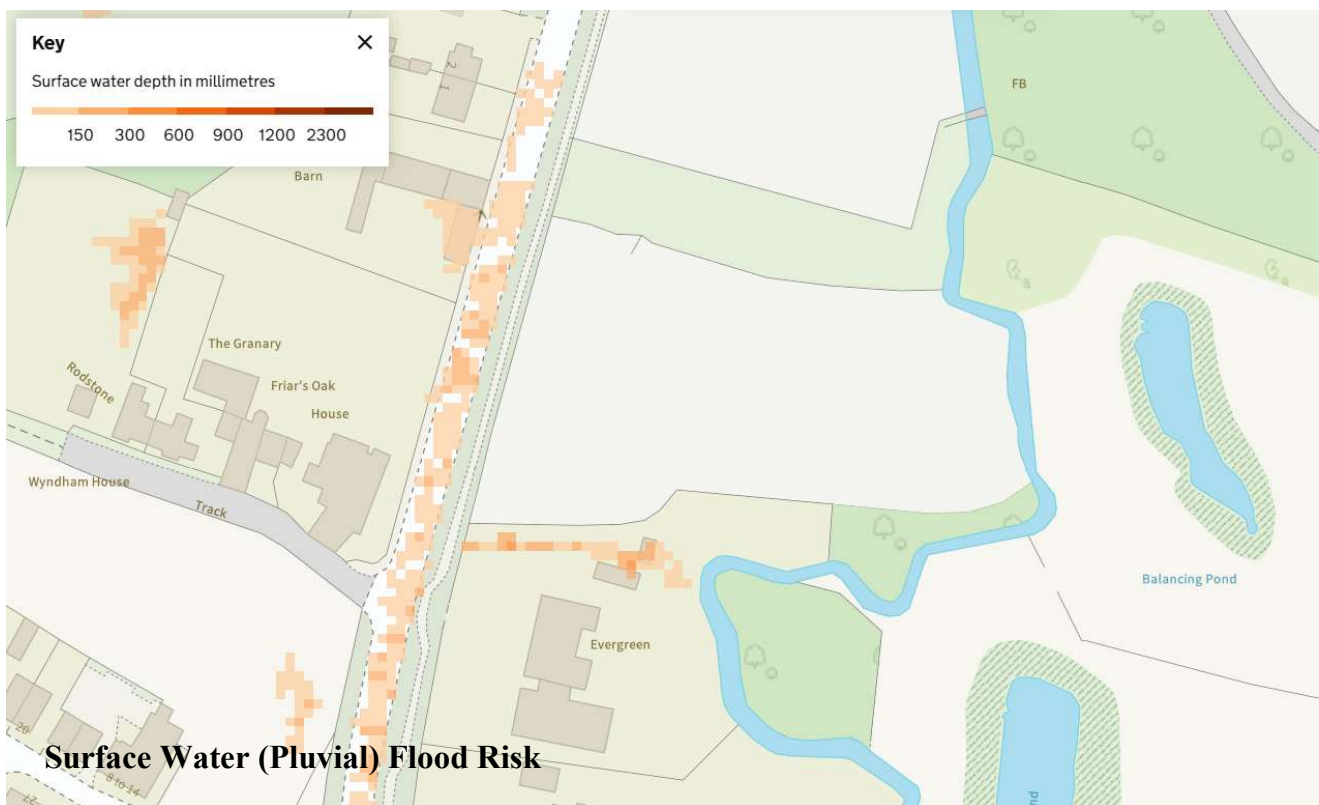
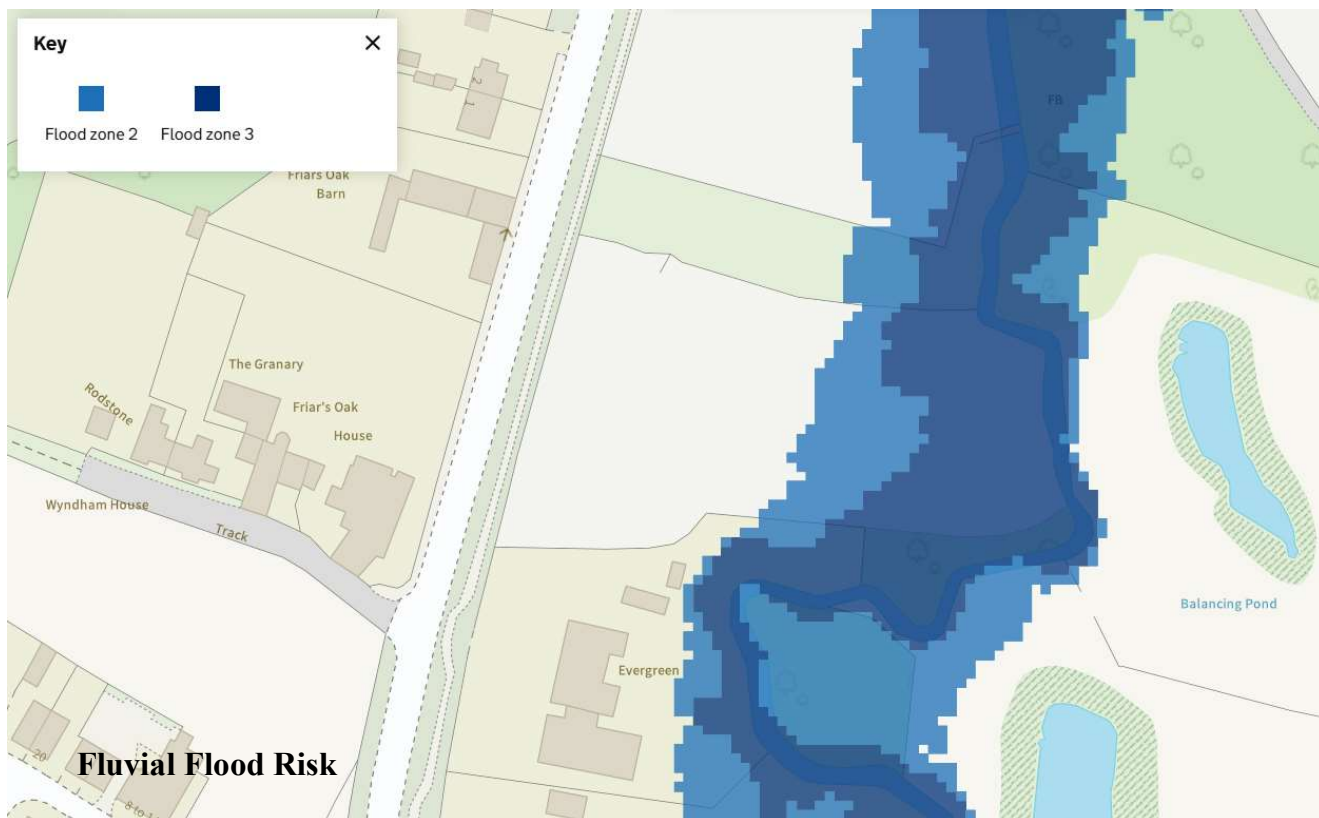
Figure 3: Suggested Drainage Arrangement

Scale as shown

August 2026

APPENDIX B

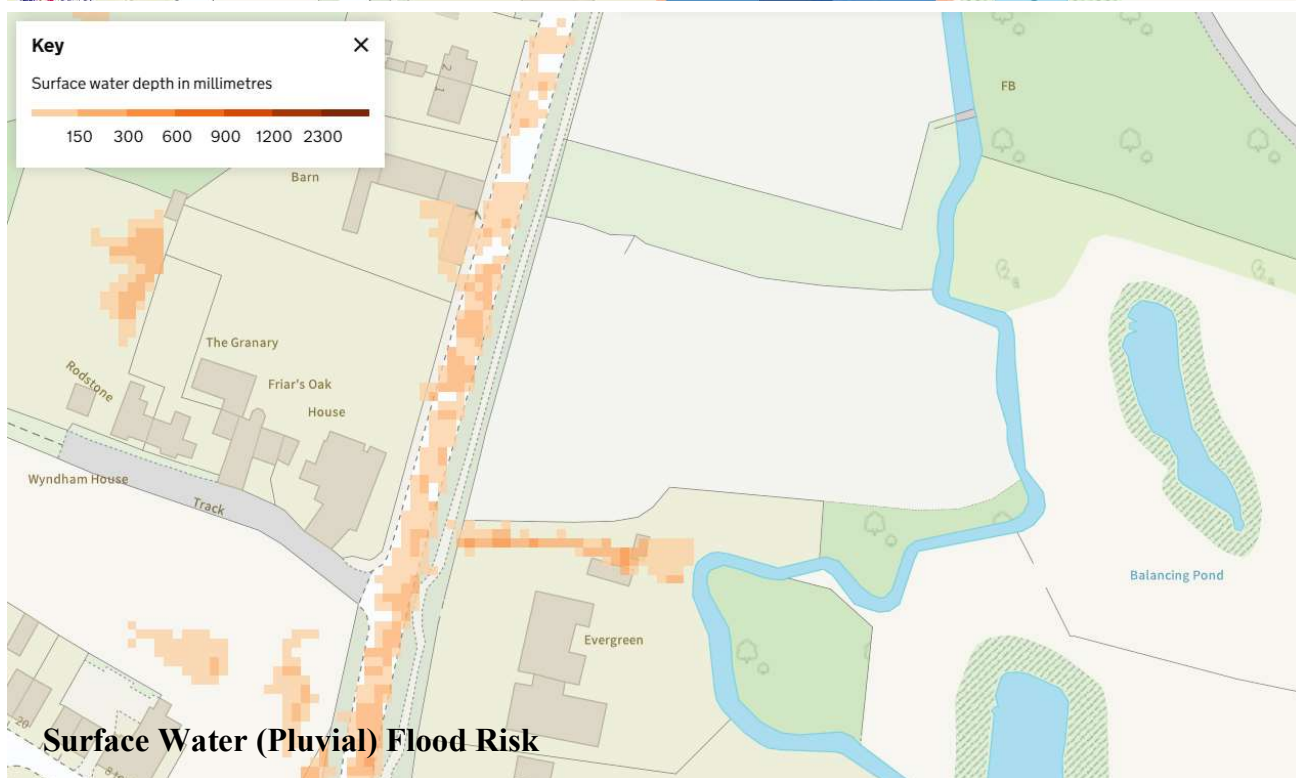
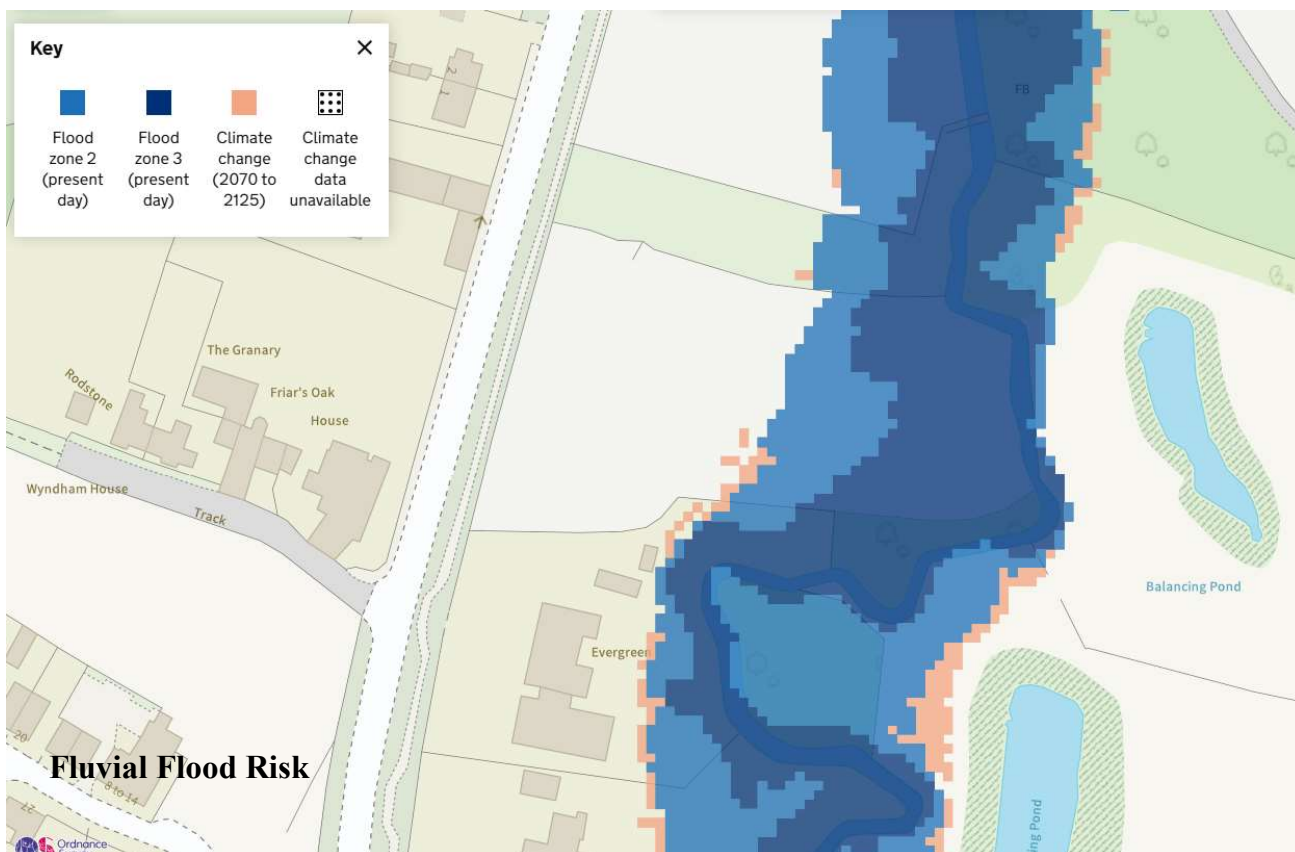
FLOOD RISK MAPPING



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 WEST SUSSEX, BN6 9NA
Flood Maps (Current)

Scale as shown

June 2026



PHILIP AND JULIE HARRIS
 LAND ADJOINING EVERGREEN, LONDON ROAD, HASOCKS
 WEST SUSSEX, BN6 9NA
Flood Maps (2070 - 2125)

Scale as shown

June 2026

APPENDIX C

SEWER PLANS



(c) Crown copyright and database rights 2026 Ordnance Survey AC0000808122 Scale: 1:1250 Date: 15/06/26 Wastewater Plan A4
 Data updated: 22/05/26 Map Centre: 530277,116717 Our Ref: 2148595 - 1 Powered by digdat

malcolm@easltd.co.uk

Land north of Ever



The positions of pipes shown on this plan are believed to be correct, but Southern Water Services Ltd accept no responsibility in the event of inaccuracy. The actual positions should be determined on site. This plan is produced by Southern Water Services Ltd (c) Crown copyright and database rights 2026 Ordnance Survey AC0000808122. This map is to be used for the purposes of viewing the location of Southern Water plant only. Any other uses of the map data or further copies is not permitted.

WARNING: BAC pipes are constructed of Bonded Asbestos Cement.

WARNING: Unknown (UNK) materials may include Bonded Asbestos Cement.

[illegible]

APPENDIX D

OUTPUT FROM THE MASTERDRAIN MODEL



MasterDrain
HY 11.0

Environmental Assessment Services Ltd

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email: info@easltd.co.uk

Job No.	893		
Sheet no.	1		
Date	25/08/26		
By	MDM	Checked	Reviewed

Project **Land adjacent to Evergreen**
Title **Hydrological data for Hassocks FEH**

Data:-

FSR values

Location = Hassocks FEH Grid reference = TQ3016
M5-60 (mm) = 20.4 SAAR (mm/yr) = 911
r = 0.35 Soil = 0.45
Long reference = 530116 Area = England and Wales
Hydrological area = 7 Hydrological zone = 1

Soil classification for WRAP type 4

Clayey, or loamy over clayey soils with an impermeable layer at shallow depth.

FEH values

c = -0.027 d1 = 0.383 d2 = 0.311
d3 = 0.433 e = 0.316 f = 2.467



MasterDrain
HY 11.0

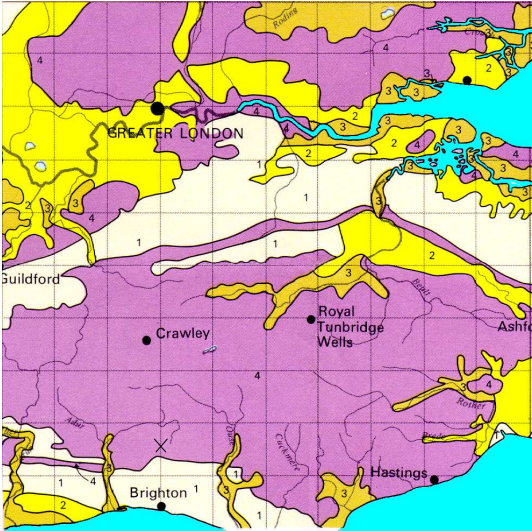
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Job No. 893		
Sheet no. 2		
Date 25/08/26		
By MDM	Checked	Reviewed

Project **Land adjacent to Evergreen**
Title **Hydrological data for Hassocks FEH**



<- Soil

M5-60 ->



<- SAAR

r x 100 ->



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MasterDrain
SW

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Job No. 893
Sheet no. 1
Date 25/08/26
By MDM
Checked
Reviewed

Project Land adjacent to Evergreen, London Road, Hassocks
Title Hydrograph storage analysis (Winter profile) for Hassocks FEH

Data:-

FEH Hydrology:-

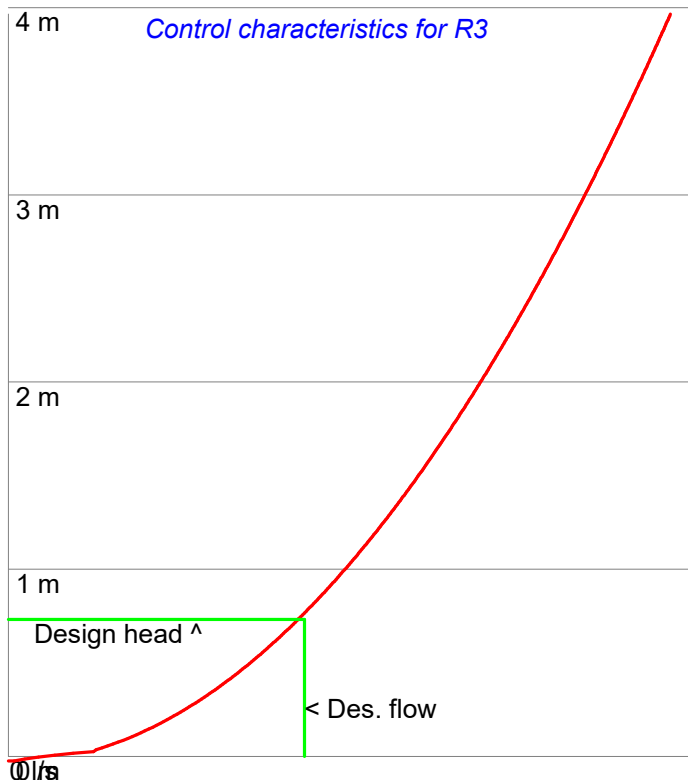
Location = Hassocks FEH Grid reference = TQ3016
 c = -0.0270 d1 = 0.3830 d2 = 0.3110
 d3 = 0.4330 e = 0.3160 f = 2.4670
 Return period = 100 Climate change = 45%

Percentage runoff = 100.0% (manual setting)

Imperv. area = 1500 m² Pervious area = 0 m²
 Total area = 1500 m² Equiv area = 1500 m² (Tot. area x % runoff).
 Total runoff = 208.7 m³ Discharge rate = 2.000 l/s

Design Head = 0.8m Peak flow = 1.99 l/s
 Control device = R3 Orifice diam = 41.8 mm
 Max. calc. depth = 0.8 m Available depth = 0.0 m³
 Mean discharge = 1.12 l/s

Available system storage = 0.00 m³ under a system plane at the design head level.
 Offline storage = 0.0 m³
 Total storage = 165.0 m³ Peak input flow = 12.08 l/s



Head (m)	Flow (l/s)	Head (m)	Flow (l/s)
0.01	0.04	2.01	3.17
0.05	0.44	2.05	3.20
0.10	0.71	2.10	3.24
0.15	0.87	2.15	3.28
0.20	1.00	2.20	3.32
0.25	1.12	2.25	3.35
0.30	1.22	2.30	3.39
0.35	1.32	2.35	3.43
0.40	1.41	2.40	3.46
0.45	1.50	2.45	3.50
0.50	1.58	2.50	3.54
0.55	1.66	2.55	3.57
0.60	1.73	2.60	3.61
0.65	1.80	2.65	3.64
0.70	1.87	2.70	3.67
0.75	1.94	2.75	3.71
0.80	2.00	2.80	3.74
0.85	2.06	2.85	3.77
0.90	2.12	2.90	3.81
0.95	2.18	2.95	3.84
1.00	2.24	3.00	3.87
1.05	2.29	3.05	3.91
1.10	2.35	3.10	3.94
1.15	2.40	3.15	3.97
1.20	2.45	3.20	4.00
1.25	2.50	3.25	4.03
1.30	2.55	3.30	4.06
1.35	2.60	3.35	4.09
1.40	2.65	3.40	4.12
1.45	2.69	3.45	4.15
1.50	2.74	3.50	4.18
1.55	2.78	3.55	4.21
1.60	2.83	3.60	4.24
1.65	2.87	3.65	4.27
1.70	2.92	3.70	4.30
1.75	2.96	3.75	4.33
1.80	3.00	3.80	4.36
1.85	3.04	3.85	4.39
1.90	3.08	3.90	4.42
1.95	3.12	3.95	4.44
2.00	3.16	4.00	4.47

Calculation data provided by Crown Water Ltd, SL5 7NT



MasterDrain
SW

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Job No.	893		
Sheet no.	2		
Date	25/08/26		
By	MDM	Checked	Reviewed

Project	Land adjacent to Evergreen, London Road, Hassocks
Title	Hydrograph storage analysis (Winter profile) for Hassocks FEH

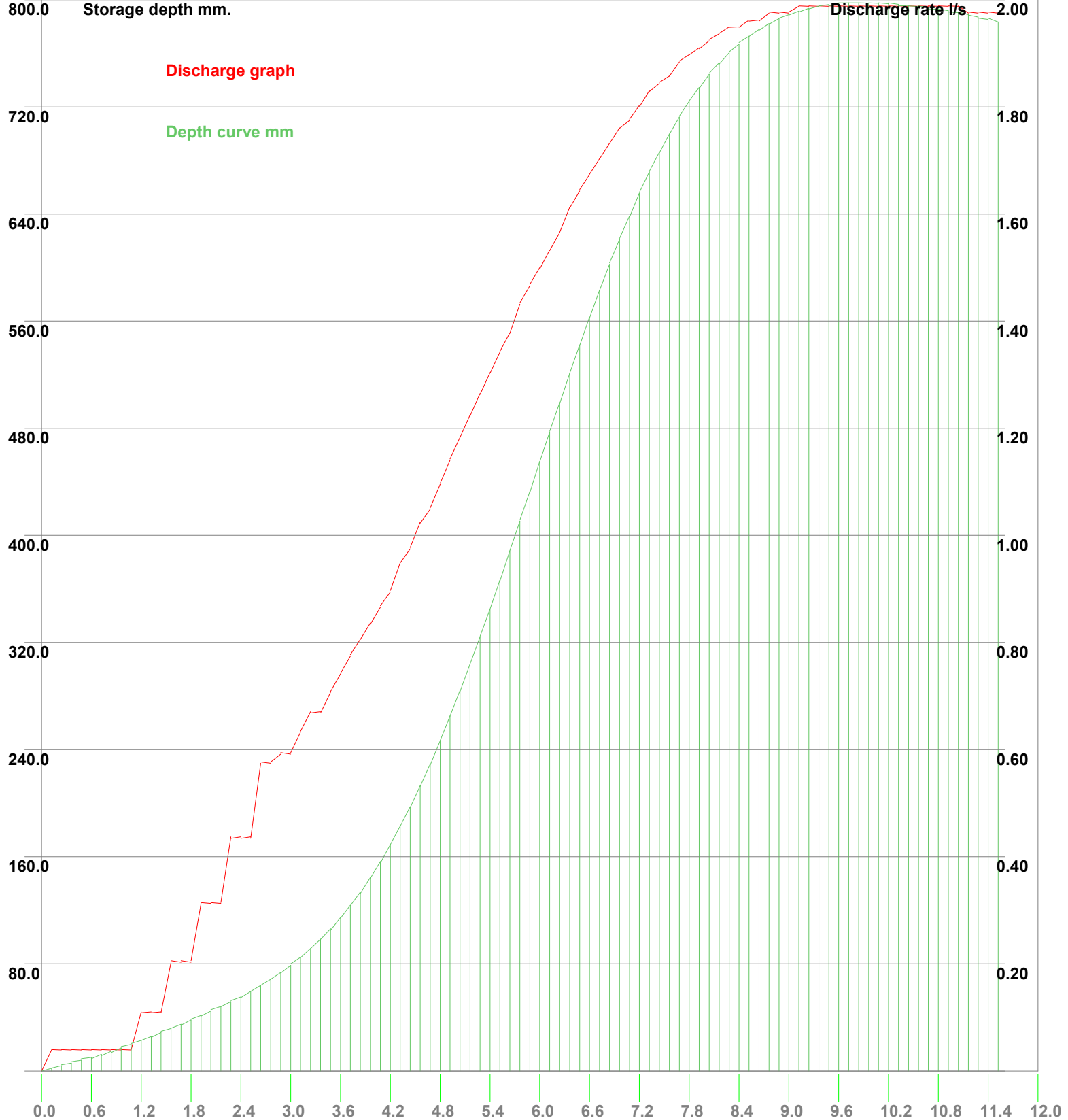
Storage curves for a 12 hours storm.

Storage depth mm.

Discharge rate l/s

Discharge graph

Depth curve mm





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Job No.	893		
Sheet no.	3		
Date	25/08/26		
By	MDM	Checked	Reviewed
Title			

Project Land adjacent to Evergreen, London Road, Hassocks

Title Hydrograph storage analysis (Winter profile) for Hassocks FEH

Incremental rainfall figures.

Storm Mins	Storage Depth mm	Control Flow l/s	Storm Mins	Storage Depth mm	Control Flow l/s
7.2	2.0	0.04	367.2	477.2	1.53
14.4	4.0	0.04	374.4	499.2	1.57
21.6	6.0	0.04	381.6	521.0	1.61
28.8	8.1	0.04	388.8	542.3	1.64
36.0	10.2	0.04	396.0	563.0	1.67
43.2	12.4	0.04	403.2	583.1	1.70
50.4	14.8	0.04	410.4	602.4	1.73
57.6	17.3	0.04	417.6	620.9	1.76
64.8	20.0	0.04	424.8	638.7	1.77
72.0	22.8	0.11	432.0	655.6	1.80
79.2	25.7	0.11	439.2	671.5	1.83
86.4	28.7	0.11	446.4	686.2	1.84
93.6	31.9	0.20	453.6	699.8	1.86
100.8	34.9	0.20	460.8	712.5	1.88
108.0	38.1	0.20	468.0	724.1	1.90
115.2	41.5	0.31	475.2	734.6	1.91
122.4	44.8	0.31	482.4	744.3	1.92
129.6	48.2	0.31	489.6	752.9	1.94
136.8	51.8	0.44	496.8	760.4	1.95
144.0	55.4	0.44	504.0	766.9	1.95
151.2	59.5	0.44	511.2	772.7	1.96
158.4	63.9	0.57	518.4	777.8	1.96
165.6	68.5	0.57	525.6	782.2	1.97
172.8	73.5	0.59	532.8	785.9	1.97
180.0	79.0	0.59	540.0	788.9	1.97
187.2	84.9	0.63	547.2	791.5	1.99
194.4	91.4	0.67	554.4	793.6	1.99
201.6	98.5	0.67	561.6	795.2	1.99
208.8	106.3	0.71	568.8	796.4	1.99
216.0	114.7	0.74	576.0	797.2	1.99
223.2	123.8	0.77	583.2	797.6	1.99
230.4	133.7	0.81	590.4	797.7	1.99
237.6	144.7	0.84	597.6	797.5	1.99
244.8	156.6	0.87	604.8	797.3	1.99
252.0	169.2	0.89	612.0	797.0	1.99
259.2	182.9	0.95	619.2	796.5	1.99
266.4	197.5	0.97	626.4	795.8	1.99
273.6	213.0	1.02	633.6	794.9	1.99
280.8	229.4	1.05	640.8	794.0	1.99
288.0	246.8	1.10	648.0	793.0	1.99
295.2	265.1	1.14	655.2	791.8	1.99
302.4	284.2	1.18	662.4	790.3	1.99
309.6	303.9	1.22	669.6	788.8	1.97
316.8	324.2	1.26	676.8	787.1	1.97
324.0	345.1	1.30	684.0	785.3	1.97
331.2	366.5	1.34	691.2	783.4	1.97
338.4	388.4	1.38	698.4	781.4	1.97
345.6	410.6	1.43	705.6	779.3	1.96
352.8	432.8	1.47	712.8	777.3	1.96
360.0	455.0	1.50	720.0	775.2	1.96

Using the Get Max button causes the program to step through a series of storm durations until a maximum volume is obtained.

Each duration is sampled 600 times and the results recorded. The storm durations (hrs) are:-

0.25, 0.5, 1, 2, 3, 4, 5, 6, 7, 8, 9, 10, 12, 15, 18, 20, 24, 30, 36, 42, 48, 54, 60, 66, 72, 84, 96, 120, 150, 175, 200, 250, 300, 375, 500, 750, 1000, 1250, 1500, 1570, 2000, 2500, 3000, 3500, 4000

It should be noted that the six hour storm frequently requested rarely demonstrates the worst case for storage.



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Job No.	893		
Sheet no.	4		
Date	25/08/26		
By	MDM	Checked	Reviewed

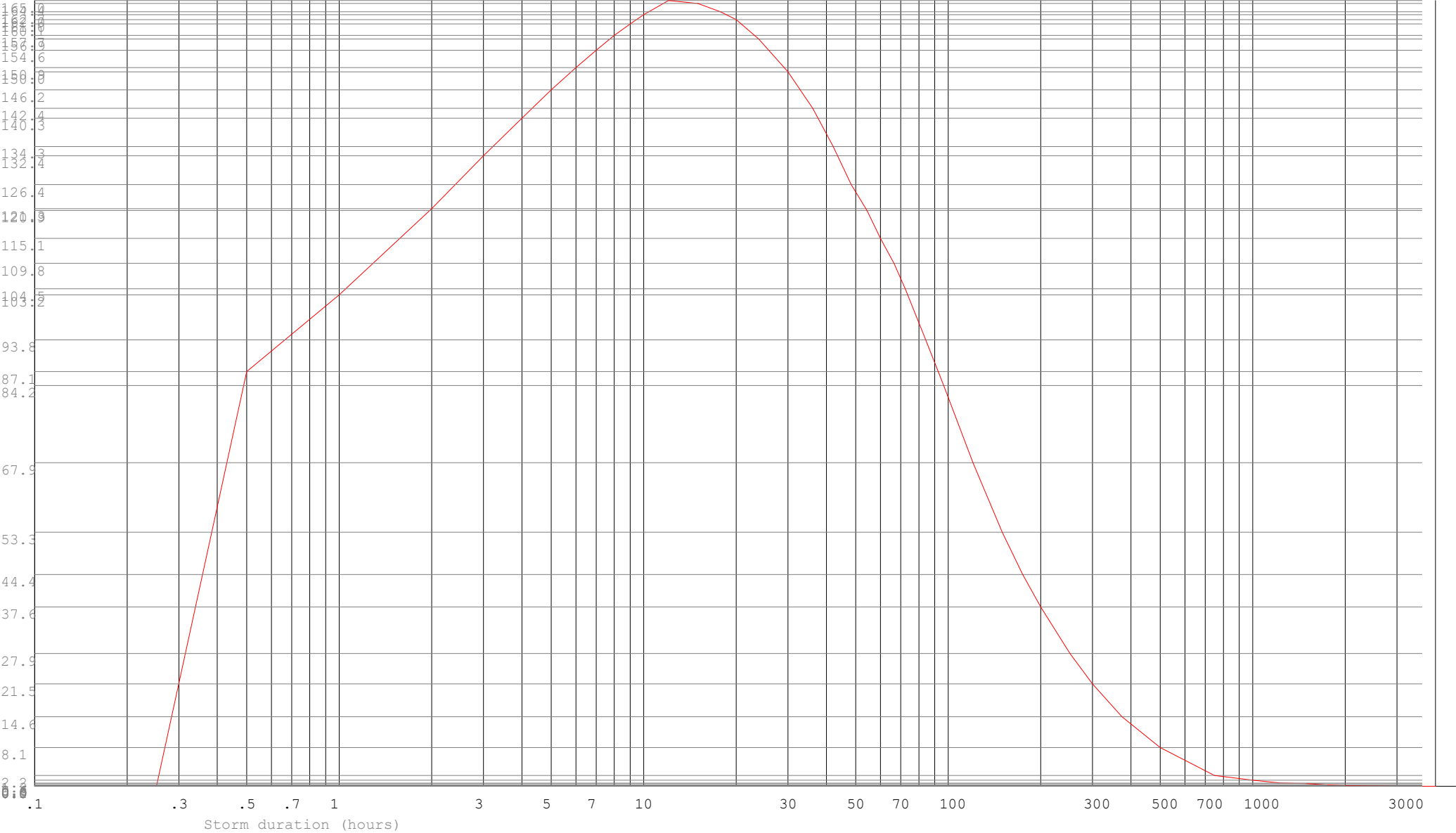
MasterDrain
SW

Project Land adjacent to Evergreen, London Road, Hassocks

Title Hydrograph storage analysis (Winter profile) for Hassocks FEH

Sequential storage volume at specific storm durations.

m³



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				Sheet no. 1	
				Date 26/08/26 10:27	
Project Land adjacent to Evergreen, Hassocks				By MDM Checked Reviewed	
Title Manhole printout for HASSOCKS FEH.SW					

Manhole ref.	X ref	Y ref	Form	Cham diam. or length	Rect. width	Chamb. height	Shaft height	MH Volume	Exit Diam	Exit Crown	Exit Invert	Chamb const	Slab level	Cover Level	Dwnstr MH
SW1	68.01	156.76		0	0			0.00	225	34.750	34.500	N/A	34.450	35.000	SW2
SW2	64.42	137.41		0	0			0.00	300	34.625	34.300	N/A	34.250	35.000	SW3
SW3	67.81	134.02		0	0			0.00	375	34.650	34.250	N/A	34.200	35.000	SW4
SW4	63.42	115.08		0	0			0.00	375	34.450	34.050	N/A	34.000	35.000	SW6
SW5	55.44	94.53		0	0			0.00	225	34.750	34.500	N/A	34.450	35.000	SW6
SW6	59.03	113.08		0	0			0.00	375	34.400	34.000	N/A	33.950	35.000	Atten
Atten	77.38	108.29		0	0			0.00	600	33.825	33.200	N/A	33.150	34.700	F7

All manholes to be as per 'Sewers for Adoption'

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Project	Land adjacent to Evergreen, Hassocks			By MDM	Checked	Reviewed
Title						

MH = SW1	MH = SW2	MH = SW3	MH = SW4	MH = SW5	MH = SW6	MH = Atten
a=head	a=head	a=head	a=head	a=head	a=head	a=head
b = 225	b = 300	b = 375	b = 375	b = 225	b = 375	b = 600
					c = 0	

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				Sheet no. 3	
				Date 26/08/26 10:27	
Project Land adjacent to Evergreen, Hassocks		By MDM	Checked	Reviewed	
Title Manhole printout for HASSOCKS FEH.SW					

These explanatory notes should be read in conjunction with the Manhole printout

- 1) Manhole ref - the reference for the manhole in question
- 2) Form - either circular or rectangular
- 3) Chamber diam or length - diameter of chamber if circular, or length if rectangular - in metres
- 4) Rectangular width - if form is rectangular, this gives the width of the chamber
- 5) Chamber height - height of chamber from invert
- 6) Shaft height - height of access shaft if applicable, from top of chamber to ground level
- 7) Manhole volume - cubic capacity of manhole chamber
- 8) Exit diameter - diameter of pipe leaving this manhole
- 9) Exit crown - crown level of pipe leaving this manhole
- 10) Exit invert - invert level of pipe leaving this manhole
- 11) Chamb const - manhole construction :-
 - N/A - not specified
 - Conc - concrete rings
 - Poly - polypropylene rings surrounded by selected fill
 - Poly/conc - polypropylene rings surrounded by concrete
 - Brick - brick built with waterproof render
 - In situ - concrete cast in situ
- 12) Slab level - surface level of base slab (if no sump, this is placed at 50mm below exit invert to allow levels manipulation).
- 13) Manhole diameters of 9999 have exceeded the sizes available in SfA
- 14) Manhole diameters may have to be modified due to the number of branches



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Project **Land adjacent to Evergreen, Hassocks**

Title **Surcharge calcs (Sized at 30 yrs storm) for HASSOCKS FEH.SW CCF = 45%**

Surcharged outlet (1.000)
Return period = 100 yrs
Climate change factor = 45

PEAK hydrograph values printed
Storm duration = 15 mins
No offline storage

Mean rain intens. 296.00 mm/hr
Storm profile = Summer
Using FEH data

Peak rain intens. 1160.32 mm/hr
Sample period = 7.5 secs.

Entry No.	SECT. No.	MANHOLE REF	PIPE CAPACITY l/s	RATE FLOW l/s	PIPE SIZE mm	CHAMBER DIAM/LxW mm	INVERT LEVEL m	WATER LEVEL m	GRND LEVEL m	SURCHARGE		EXCESS FLOW l/s	FLOODED VOL m³	DRAINED AREA (m²) :x FACTOR	STATUS	
1	I	1.01	SW1	52.4	33.6	225	0x0	34.50	34.64	35.00	0.64	0.14	0.00	0.000	238	OK
2	I	1.02	SW2	113.3	67.2	300	0x0	34.30	34.47	35.00	0.59	0.17	0.00	0.000	475	OK
3	I	1.03	SW3	203.0	100.9	375	0x0	34.25	34.44	35.00	0.50	0.19	0.00	0.000	712	OK
4	I	1.04	SW4	204.1	134.5	375	0x0	34.05	34.44	35.00	0.66	0.39	0.00	0.000	950	Surch.
5	B	2.01	SW5	84.6	33.6	225	0x0	34.50	34.59	35.00	0.40	0.09	0.00	0.000	238	OK
6	I	1.05	SW6	409.8	201.7	375	0x0	34.00	34.39	35.00	0.49	0.39	0.00	0.000	1425	Surch.
7	I	1.06	Atten	755.7	201.7	600	0x0	33.20	34.39	34.70	0.27	1.19	0.00	0.000	1425	Surch.



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Date 26/08/26		
By MDM	Checked	Reviewed

Project	Land adjacent to Evergreen, Hassocks
Title	Surcharge calcs (Sized at 30 yrs storm) for HASSOCKS FEH.SW CCF = 45%

Notes

Printout headings

- | | | |
|--|---|---|
| 1) Entry no - position in file | 2) Section no - pipe identifier | 3) Manhole ref - Manhole identifier |
| 4) Pipe cap - full bore capacity of that pipe | 5) Rate of flow - calculated flow rate (l/s) ± = flow restrictor. | 6) Pipe diam - outlet pipe diameter (mm) |
| 7) Chamber diam - chamber diam. at base of MH, or Manhole length x width | | 8) Static level - 1m surcharge level |
| 9) Invert level - invert level of manhole | 10) Water level - calculated peak water level. | 11) Grnd level - ground / cover level |
| 12) Surch. fract - calc.flow/pipe capacity | 13) Surch. depth - surcharge level above soffit | 14) Overflow - surcharged flow rate (l/s) |
| 15) Flooded vol - volume of water above cover | 16) Upstrm Vol - upstream pipe vol to previous manhole(s) | 17) Status - OK - outlet not surcharged |
| 18) Status - Surcharged - outlet surcharged | 19) Status - Warning - water level within 299mm of cover level | 20) Status - Flooded - cover over-topped |

Title box

Hydrograph data

- | | | |
|---|--------------------------------------|--|
| 1) Ret. period - that used to calculate profile | 2) Duration - length of storm (mins) | 3) Profile - either Winter (75%) or Summer (50%) |
|---|--------------------------------------|--|

Flood volumes

Check that the upstream storage of the manhole is adequate to take the flood volume - see Upstrm Vol above.



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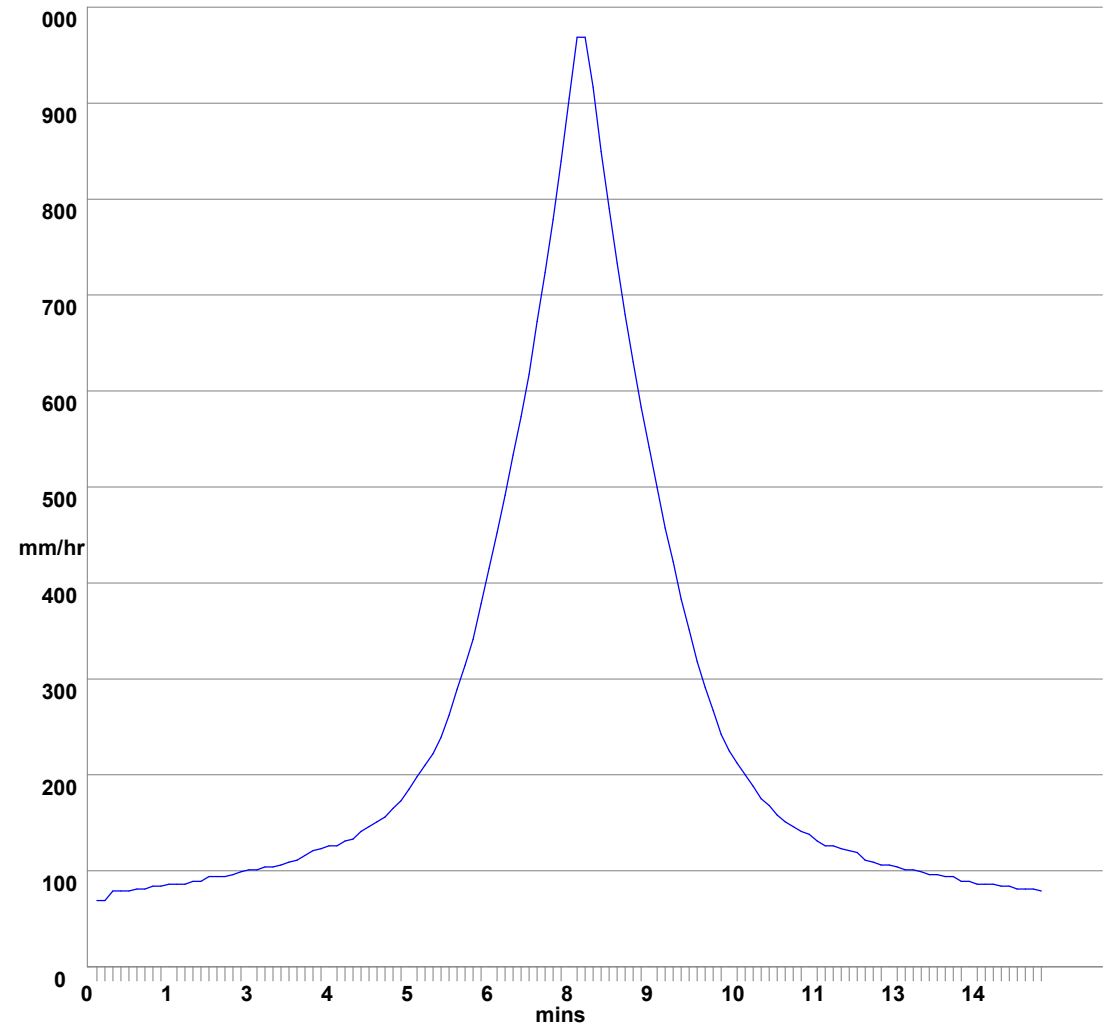
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SW

Project Land adjacent to Evergreen, Hassocks	By MDM	Checked	Reviewed
Title Surcharge calcs (Sized at 30 yrs storm) for HASSOCKS FEH.SW CCF = 45%			

Time mins	Rain mm/hr	Time mins	Rain mm/hr	Time mins	Rain mm/hr
0:08	69.13	5:08	197.52	10:08	212.33
0:15	69.13	5:15	209.86	10:15	199.99
0:22	79.01	5:22	222.21	10:22	187.64
0:30	79.01	5:30	239.49	10:30	175.30
0:38	79.01	5:38	261.71	10:38	167.89
0:45	81.48	5:45	288.87	10:45	158.02
0:52	81.48	5:52	313.56	10:52	150.61
1:00	83.95	6:00	340.72	11:00	145.67
1:08	83.95	6:08	377.76	11:08	140.73
1:15	86.41	6:15	414.79	11:15	138.26
1:22	86.41	6:22	451.83	11:22	130.86
1:30	86.41	6:30	491.33	11:30	125.92
1:38	88.88	6:38	533.30	11:38	125.92
1:45	88.88	6:45	572.81	11:45	123.45
1:52	93.82	6:52	617.25	11:52	120.98
2:00	93.82	7:00	671.57	12:00	118.51
2:08	93.82	7:08	723.41	12:08	111.10
2:15	96.29	7:15	777.73	12:15	108.64
2:22	98.76	7:22	839.46	12:22	106.17
2:30	101.23	7:30	903.65	12:30	106.17
2:38	101.23	7:38	967.84	12:38	103.70
2:45	103.70	7:45	967.84	12:45	101.23
2:52	103.70	7:52	916.00	12:52	101.23
3:00	106.17	8:00	849.33	13:00	98.76
3:08	108.64	8:08	790.08	13:08	96.29
3:15	111.10	8:15	733.29	13:15	96.29
3:22	116.04	8:22	678.97	13:22	93.82
3:30	120.98	8:30	629.59	13:30	93.82
3:38	123.45	8:38	582.68	13:38	88.88
3:45	125.92	8:45	540.71	13:45	88.88
3:52	125.92	8:52	498.74	13:52	86.41
4:00	130.86	9:00	456.76	14:00	86.41
4:08	133.33	9:08	422.20	14:08	86.41
4:15	140.73	9:15	382.69	14:15	83.95
4:22	145.67	9:22	350.60	14:22	83.95
4:30	150.61	9:30	318.50	14:30	81.48
4:38	155.55	9:38	291.34	14:38	81.48
4:45	165.42	9:45	266.65	14:45	81.48
4:52	172.83	9:52	241.96	14:52	79.01
5:00	185.17	10:00	224.68	15:00	79.01

Hydrograph profile derived from data in the Flood Studies Report

Return period= 100 yrs Duration= 15 mins Profile - summer





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SW

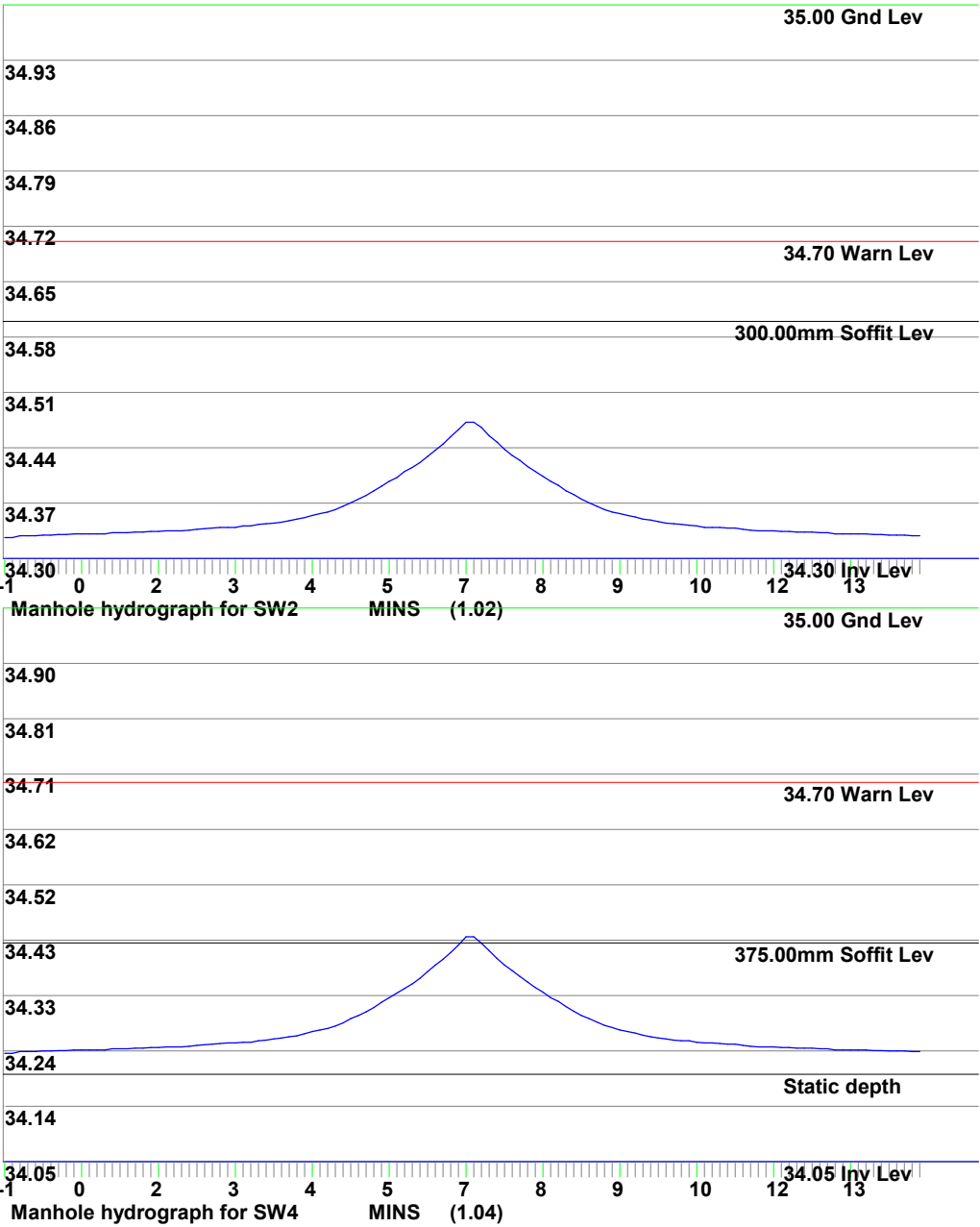
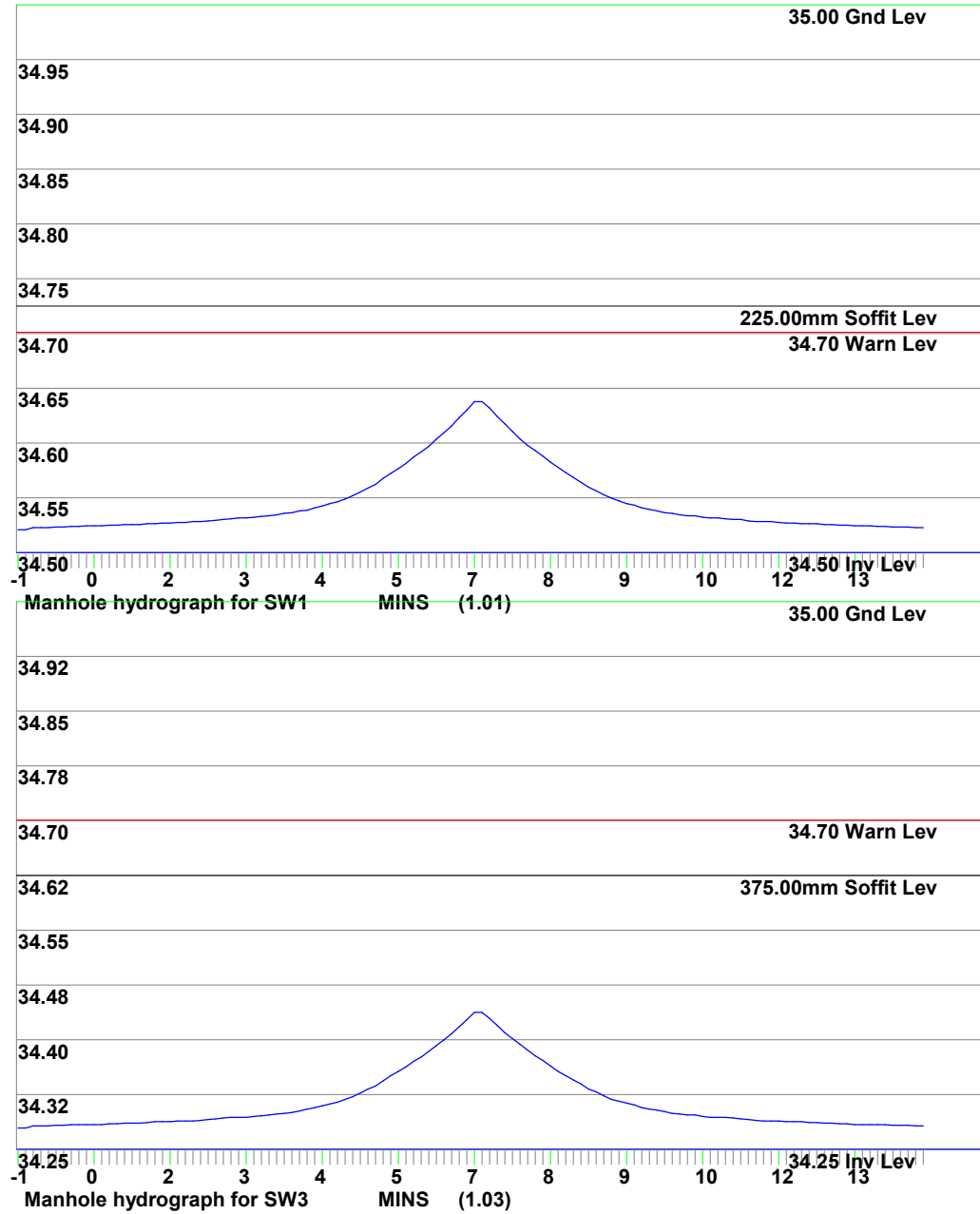
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Project	Land adjacent to Evergreen, Hassocks
Title	Surcharge calcs (Sized at 30 yrs storm) for HASSOCKS FEH.SW CCF = 45%





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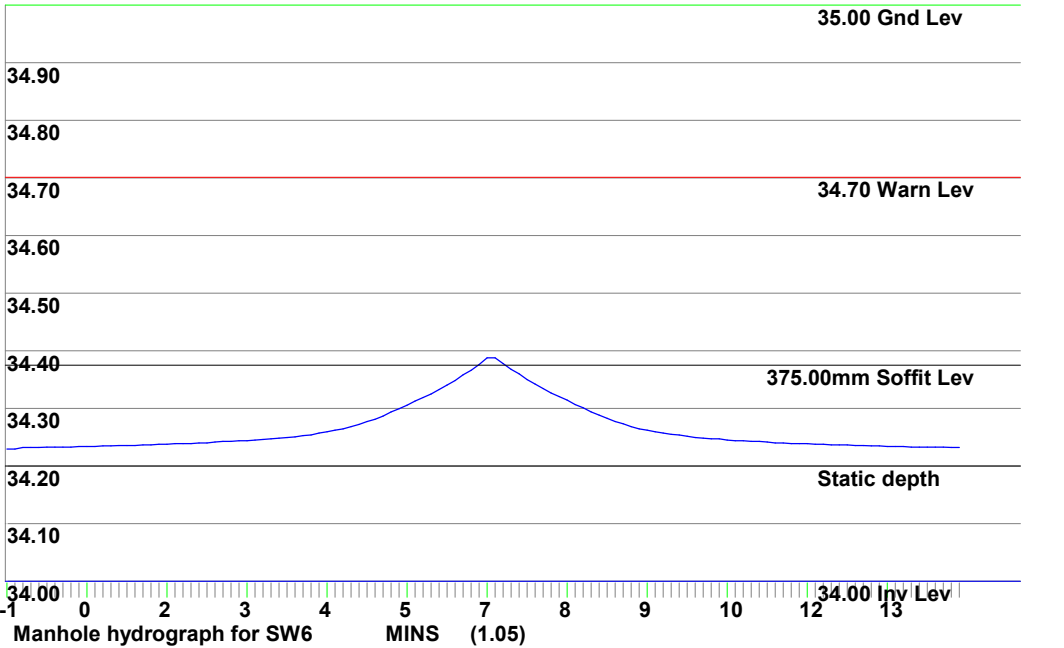
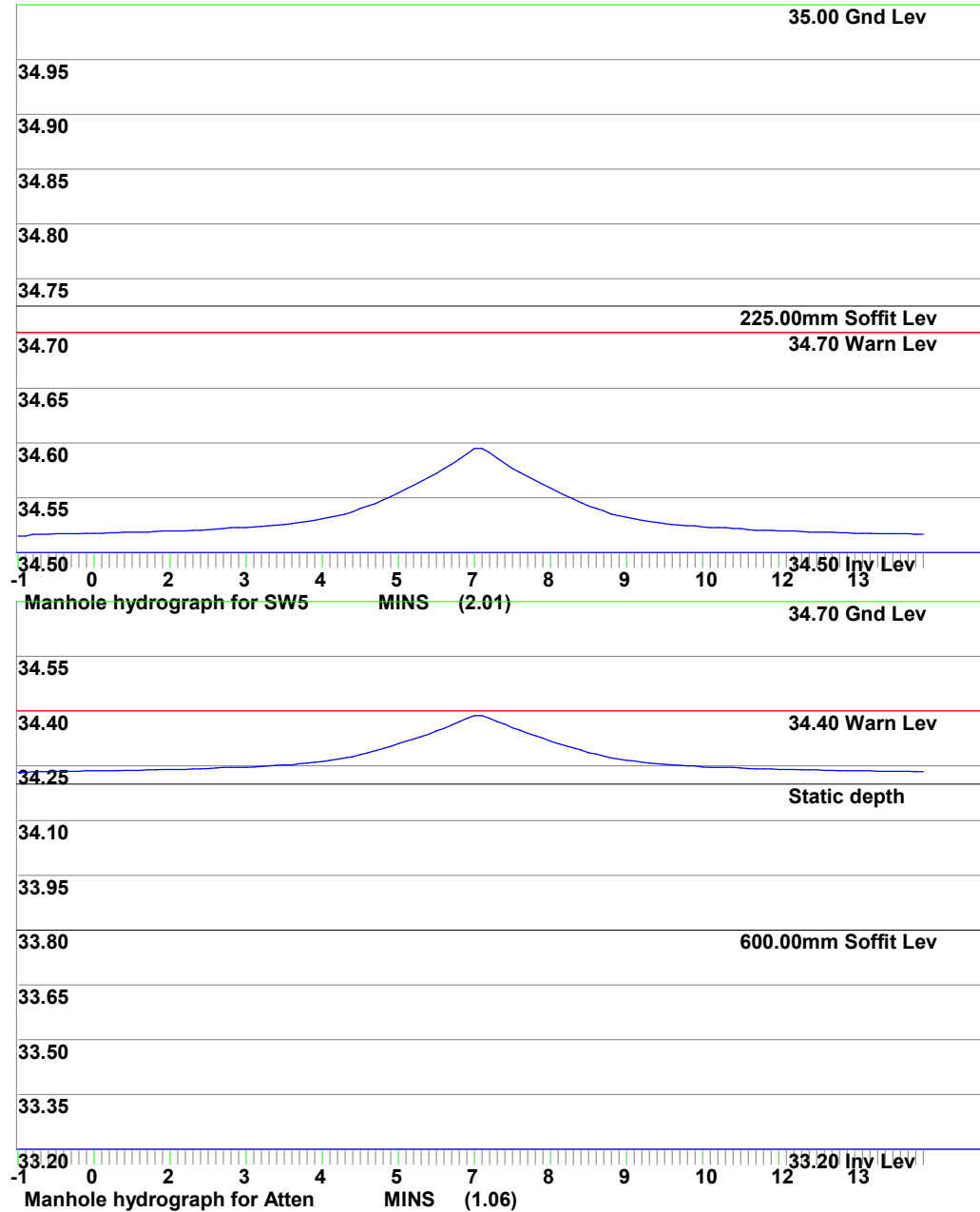
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Project	Land adjacent to Evergreen, Hassocks
Title	Surcharge calcs (Sized at 30 yrs storm) for HASSOCKS FEH.SW CCF = 45%





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SW

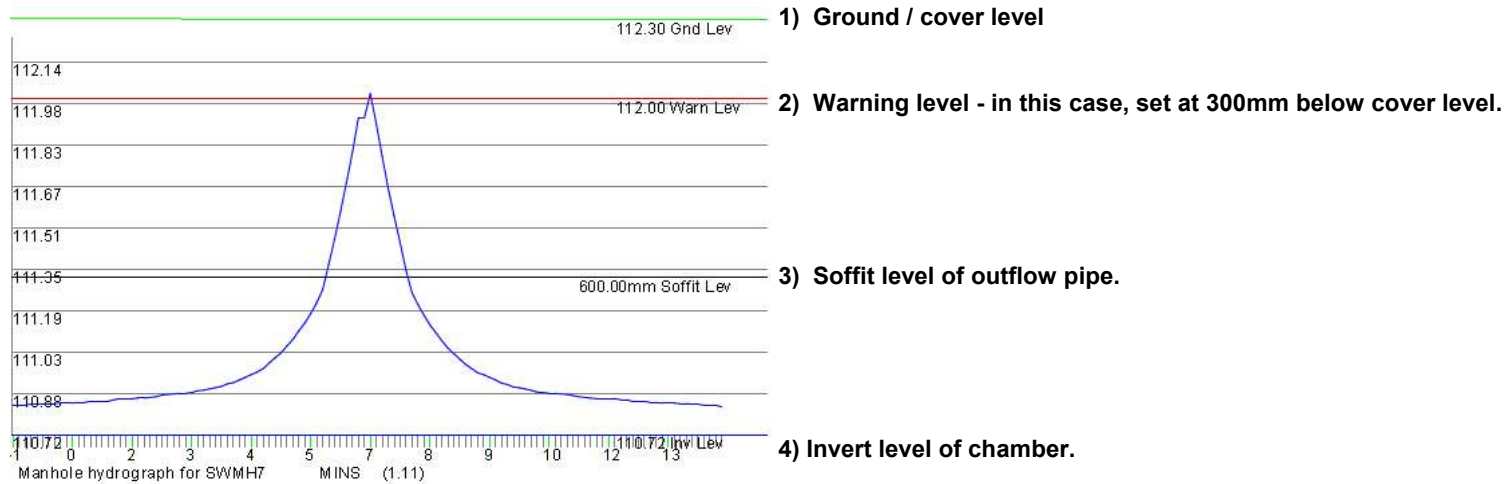
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Date 26/08/26	
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Project Land adjacent to Evergreen, Hassocks
Title Surcharge calcs (Sized at 30 yrs storm) for HASSOCKS FEH.SW CCF = 45%



Notes

- Lower section of the graph shows the water depth filling the channel. Channel is assumed to be a full pipe diameter in depth.
- Upper section of the graph shows the water depth filling the chamber. Chamber has a greater width/diameter than the channel, so increases in depth are proportionally less.
- The top of the graph clips the warning level and would be marked thus on the printout.
- In many cases the invert of the offline storage is required to enter at the channel soffit level, meaning that the pipe will still surcharge but flooding risk reduced.
- The diagram above is a general one and is not part of the current calculation.



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Foul 11.5

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				Date 26/08/26		
Project Land adjacent to Evergreen, Hassocks				By MDM	Checked	Reviewed
Title Manhole printout for LAND AT EVERGREEN, HASSOCKS.FL						

Manhole ref.	Form	Cham diam. or length	Rect. width	Chamb. height	Shaft height	MH Volume	Ent. Diam	Ent. Crown	Ent. Invert	Exit Diam	Exit Crown	Exit Invert	Chamb const	Cover Level	Dwnstr MH
F1	circ	450	N/A	0.50		0.08	Head	----	----	100	34.625	34.500	N/A	35.000	F2
F2	circ	450	N/A	0.68		0.11	100	0.000	0.000	100	34.445	34.320	N/A	35.000	F3
F3	circ	600	N/A	0.93		0.26	100	0.000	0.000	100	34.195	34.070	N/A	35.000	2701

All manholes to be as per 'Sewers for Adoption'

Levels calculated for level inverts



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Foul 11.5

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Date 26/08/26		
By MDM	Checked	Reviewed

Project Land adjacent to Evergreen, Hassocks	By MDM	Checked	Reviewed
Title Manhole printout for LAND AT EVERGREEN, HASSOCKS.FL			

These explanatory notes should be read in conjunction with the Manhole printout

- 1) Manhole ref - the reference for the manhole in question
- 2) Form - either circular or rectangular
- 3) Chamber diam or length - diameter of chamber if circular, or length if rectangular - in metres
- 4) Rectangular width - if form is rectangular, this gives the width of the chamber
- 5) Chamber height - height of chamber from invert
- 6) Shaft height - height of access shaft if applicable, from top of chamber to ground level
- 7) Manhole volume - cubic capacity of manhole chamber
- 8) Exit diameter - diameter of pipe leaving this manhole
- 9) Exit crown - crown level of pipe leaving this manhole
- 10) Exit invert - invert level of pipe leaving this manhole
- 11) Manhole diameters of 9999 have exceeded the sizes available in Sfa
- 12) Manhole diameters may have to be modified due to the number of branches

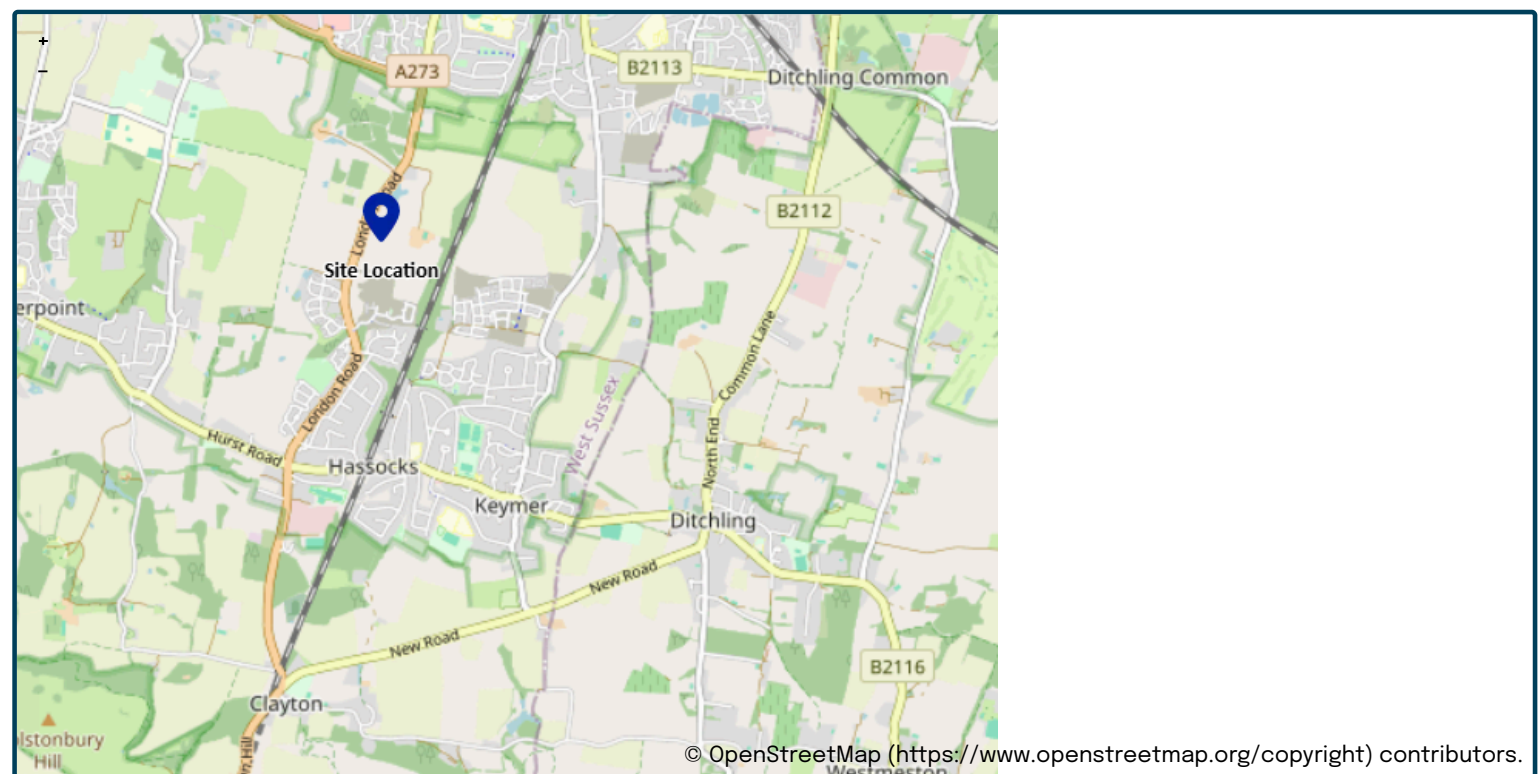
This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	25/08/2026
Calculated by	Malcolm McKemey
Reference	893
Model version	2.2.3

Location

Site name	Land adjacent to Evergreen
Site location	Hassocks



Site easting (British National Grid)	530468
Site northing (British National Grid)	116805

Site details

Total site area (ha)	0.15	ha
----------------------	-----------------------------------	----

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MORE INFO

Greenfield runoff

Method

Method

FEH statistical (2025)

FEH statistical (2025)

My value

Map value

SAAR9120 (mm)

956

mm

BFIHOST19scaled

0.643

QMed-QBar conversion

1.136

1.136

QMed (l/s)

0.5

l/s

QBar (FEH statistical 2025) (l/s)

0.5

l/s

Growth curve factors

My value

Map value

Hydrological region

7

7

1 year growth factor

0.85

2 year growth factor

0.88

10 year growth factor

1.62

30 year growth factor

2.3

100 year growth factor

3.19

200 year growth factor

3.74

Results

Method

FEH statistical (2025)

Flow rate 1 year (l/s)

0.4

l/s

Flow rate 2 year (l/s)

0.5

l/s

Flow rate 10 years (l/s)

0.8

l/s

Flow rate 30 years (l/s)

1.2

l/s

Flow rate 100 years (l/s)

1.6

l/s

Flow rate 200 years (l/s)

1.9

l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent ‘zero’ figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.3) developed by HR Wallingford and available at uksuds.com (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at [uksuds.com/terms-conditions](https://www.uksuds.com/terms-conditions) (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

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APPENDIX E

MAINTENANCE & MANAGEMENT PLAN

MAINTENANCE MANAGEMENT PLAN

E1 **Initial maintenance:**

- E1.1 On completion of the development, all chambers, pipework and the attenuation storage outlet and flow control device for the surface water drainage should be checked and cleared/de-silted as necessary. Any remaining areas of bare ground within the surface water drainage catchment should be planted/seeded or surfaced to minimise loose soil washing into the drainage system. The foul chambers and pipework should be checked at the same time
- E1.2 After the first late winter following completion of the development (after most leaf fall has occurred), all gullies and chambers should be de-silted, and the attenuation storage flow control chamber outlet cleared of debris.

E2 **Annual maintenance:**

- E2.1 Annual maintenance should generally be carried out after late winter when most leaf fall will have occurred. If siltation and debris blockage is found to be excessive, more frequent maintenance may be indicated.
- E2.2 Annual maintenance will comprise:
- Checking all chamber covers and frames for damage and fit.
 - Checking all gully gratings for damage and fit.
 - Clear attenuation storage controlled flow outlet and check for function.
 - De-silting and removing debris from all chambers and gullies/strip drains.
 - Checking the surface water outfall.

E4. **Responsibility for carrying out the Maintenance Management Plan:**

- E4.1 The developer/contractor will be responsible for the initial maintenance and the annual maintenance up to the end of any maintenance or guarantee period.
- E4.2 After the end of the maintenance or guarantee period, it will become the responsibility of the freeholders of the properties. A management committee may be formed or maintenance may be assigned to one householder, with financial recompense from the others.
- E4.3 A record of all maintenance work carried out should be kept as part of the Maintenance Management Plan.

☆ ☆ ☆ ☆ ☆ ☆ ☆

APPENDIX F

COMPLETED FLOOD RISK AND DRAINAGE CHECKLIST

FLOOD RISK AND DRAINAGE INFORMATION CHECK LIST – APPLICATION STAGE

v2.0 issued 07/11/25

This checklist should be submitted to support all planning applications in Mid Sussex District where flood risk or drainage issues are relevant. It reflects national policy, Environment Agency Standing Advice, the National standards for sustainable drainage systems (SuDS) (Defra, June 2025), and the Mid Sussex District Council Level 1 Strategic Flood Risk Assessment (SFRA) (2024 or as updated).

This checklist is designed to support applicants within the planning process provide the information required by the flood risk and drainage team for them to provide a consultation response. This is not the complete planning validation list, however much of the information is also included with the validation list.

The level of detail provided to address each information point should reflect the scale and complexity of the development.

Receipt of this information does not guarantee the flood risk and drainage team will support an application or does it prevent a request for further information. It does, however, ensure the team has sufficient information to undertake a full review and provide detailed consultation response.

Applicants should complete this checklist to confirm whether the required information has been provided to support validation. If applicants are unsure what information or level of detail is required, we strongly encourage early engagement with the flood risk and drainage team through a pre-application enquiry.

This checklist should be completed by the applicant or their consultant and submitted in support of the planning application. In the right-hand column, you should cross-reference where in your submission each item is addressed (e.g. report name, page number, or drawing reference). Failure to address mandatory items may result in your application being or delayed.

All Flood Risk Assessments and Drainage Strategies should be prepared and signed off by a suitably qualified and experienced professional, such as a qualified flood risk consultant, drainage engineer, or practitioner with demonstrable experience, with appropriate knowledge of current national policy and local SFRA requirements.

SECTION A: SITE AND APPLICATION DETAILS

Requirement	Stated Value (if applicable)	Reference (Drawing / Report Section)
Application reference number	DM/25/2046	
Site address	LAND ADJOINING EVERGREEN, LONDON ROAD, HASSOCKS, BNG 9NA	
Easting/Northing coordinates		1.1
Site area (hectares) / number of units	0.365	1.1
Description of existing development		1.2
Description of proposed development		1.3
Existing and proposed site plans / elevation plans		Figs 1, 2 & 3

Block plan (to scale)		Fig 2
Evidence demonstrating that the Sequential Test, and where applicable the Exception Test, have been applied, and successfully met, in the selection of the site (See section E below).		SECTION 6
Information on the expected lifetime of the proposed development and how that figure was estimated.		100 YEARS HOUSING
Existing and proposed vulnerability classification (as per Annex 3: Flood risk vulnerability classification)		SECTION 3

SECTION B: FLOOD RISK SCREENING

All sites must be screened against flood risk datasets to determine whether a site-specific Flood Risk Assessment (FRA) and Sequential and/or Exception Test are required.

Applicants must use the Mid Sussex District Council Level 1 Strategic Flood Risk Assessment (SFRA) (2024 or as updated), the latest Environment Agency Flood Map for Planning, and the Risk of Flooding from Surface Water (RoFSW) dataset to determine the site's flood zone, surface water risk, and any mapped functional floodplain or climate change overlays. This includes identifying whether any part of the site, access and escape route lies in:

- Flood Zone 2, 3a, or 3b (as defined by SFRA and EA data);
- High surface water flood risk areas (>3.3% AEP);
- Locations affected by future flood zones under climate change scenarios.

The SFRA mapping and guidance must be used as the primary evidence base when screening the site.

Useful data sources include:

- Environment Agency Flood Map for Planning: <https://flood-map-for-planning.service.gov.uk>
- Risk of Flooding from Surface Water map: <https://check-long-term-flood-risk.service.gov.uk>
- MSDC Strategic Flood Risk Assessment (2024 or as updated): <https://www.midsussex.gov.uk/planning-building/flood-risk-and-drainage-for-planning/> and [SFRA Interactive Web Maps](#)
- Annex 3: Flood risk vulnerability classification: <https://www.gov.uk/guidance/national-planning-policy-framework/annex-3-flood-risk-vulnerability-classification>

Requirement	Stated Value (if applicable)	Reference (Drawing / Report Section)
Fluvial Flood Zone (EA flood map for planning)		APPENDIX B
Surface water flood risk (EA risk mapping & SFRA Interactive Web Maps)		APPENDIX B
Site located in SFRA Interactive Web Maps-defined Flood Zone 3b or future Flood Zone?		SECTION 3
Historic flooding at the site (also refer to SFRA Interactive Web Maps)		SECTION 5
Climate change allowances for fluvial and surface water flood risk (2023 EA uplift)		10.5
Information on the predicted depth and level for the design flood level for all relevant sources of flood risk		APPENDIX B
Design plans showing floor levels relative to predicted flood depths (fluvial and / or surface water) with appropriate freeboard levels.	Above +35.6	SECTION 7
Groundwater flood risk		3.4

Evidence all attenuation features are inside the application boundary		FIGURE 3
Evidence of infiltration feasibility (mapping/tests) — e.g. BGS mapping or BRE365 site-specific tests		SECTION 10 RECOMMENDATIONS
Groundwater levels		NOT KNOWN
Contaminated Land		UNLIKELY
Surface water drainage constraints plan (e.g. RPZs, buffers, utilities, topography, sufficient offsets)		FIGURE 3
Drainage strategy plan showing SuDS layout and discharge points (including above- and below-ground features)		FIGURE 3
Drainage calculations provided that demonstrate design standard met (1 in 100-year + climate change) — include storm duration and EA uplift applied. A CV value of 1.0 and an Urban Creep allowance, where applicable.		APPENDIX D
Outfall agreed in principle by water/sewer authority (if relevant)		11.3
Management & Maintenance Plan		APPENDIX E

SECTION D: FOUL WATER DRAINAGE STRATEGY

At application stage, the applicant should demonstrate that foul water drainage can be achieved in principle, including the intended disposal method (e.g. mains sewer or non-mains system) and confirmation of relevant constraints.

Where a non-mains system is proposed, applicants must state how the proposal complies with the Environment Agency's General Binding Rules.

Full technical evidence — such as pump design, capacity checks, or foul infiltration field testing — will be required at the condition discharge stage and must meet the criteria set out in the Detailed Drainage Design Technical Summary and Detailed Drainage Design Checklist.

Requirement	Stated Value (if applicable)	Reference (Drawing / Report Section)
Details on the site's existing foul water drainage arrangements		SECTION 11
Proposed foul drainage method		SECTION 11
Public sewer records included		APPENDIX C
Drainage constraints plan		FIGURE 3
Drainage strategy plan		FIGURE 3
Management & Maintenance Plan		APPENDIX E
If using non-mains drainage, EA General Binding Rules compliance statement		N/A

SECTION E: SEQUENTIAL AND EXCEPTION TEST EVIDENCE

The Sequential Test is required in accordance with the National Planning Policy Framework (NPPF, December 2024, paragraph 172), Planning Practice Guidance (PPG, updated August 2022), and the Mid Sussex District Council Level 1 Strategic Flood Risk Assessment (SFRA) (2024 or as updated). It must be applied to all planning applications for any proposed building, access and escape route, land-raising or other vulnerable element for:

- Development in Flood Zones 2 or 3, including residential, commercial, and mixed-use proposals, unless explicitly exempt;
- Development in Flood Zone 1 where the SFRA or other datasets identify increased flood risk from any source — including surface water, groundwater, sewer or reservoir flooding;
- Development within Flood Zone 1 and the flood map for planning shows it is at risk of flooding from surface water;
- Any part of a site falling within or adjacent to Flood Zone 3b (functional floodplain) and development is not incompatible, as defined by the SFRA;
- Development within Flood Zone 1 and the flood map for planning shows it is at increased risk of flooding from rivers or sea during its lifetime.

Important: In accordance with the Mid Sussex District Council Level 1 Strategic Flood Risk Assessment (SFRA) (2024 or as updated), all land within Flood Zone 3 should be treated as Zone 3b unless robust fluvial modelling or other site-specific evidence demonstrates otherwise. However, areas identified as ‘high risk’ in the Risk of Flooding from Surface Water (RoFSW) mapping — i.e. those with >3.3% annual probability — do not automatically fall within Zone 3b. These are based on different datasets and assumptions and should be assessed separately in the Flood Risk Assessment.

Applicants must consider both present-day and future flood risk, incorporating climate change allowances across all sources.

The applicant must demonstrate whether the Sequential Test applies and provide a supporting assessment if so.

If required, the Sequential Test must include a defined search area as agreed with the Local Planning Authority, a table of alternative sites, and justification for the site’s selection based on flood risk and policy compatibility, in line with Mid Sussex District Council guidance.

If the Exception Test is triggered, both parts must be addressed:

- Wider sustainability benefits aligned with local policy

- Demonstration that the development will be safe for its lifetime and not increase flood risk elsewhere, supported by the Flood Risk Assessment

See Mid Sussex District Council’s “Applicant Guidance Note: Applying the Sequential and Exception Tests for Windfall Sites” for full submission expectations.

Exemptions from the Sequential Test apply only to:

- Householder development (no new dwellings);
- Minor non-domestic extensions (<250m²);
- Changes of use (except to highly vulnerable uses like caravan parks);
- Sites allocated through the adopted or submission District Plan, where flood risk has not materially changed.

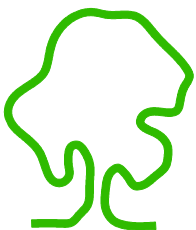
Requirement	Stated Value (if applicable)	Reference (Drawing / Report Section)
Sequential Test required		SECTION 6
Sequential Test evidence submitted		SECTIONS 3 + 6
Exception Test required		N/A
Exception Test evidence submitted		N/A

SECTION F: SUPPORTING TECHNICAL EVIDENCE

Requirement	Stated Value (if applicable)	Reference (Drawing / Report Section)
Topographic survey with ground and finished floor levels		NOT AVAILABLE
Flood modelling outputs (if FRA uses bespoke modelling)		APPENDIX D
Maps of flood hazard, depth, and velocity		APPENDIX B
SuDS design summary (for major developments — WSCC LLFA SuDS Proforma should be completed where applicable)		MINOR DEVELOPMENT
Drainage calculations		APPENDIX D
Management and maintenance plan		APPENDIX E
Contaminated Land report		NOT KNOWN

SECTION G: ADDITIONAL INFORMATION

Requirement	Stated Value (if applicable)	Reference (Drawing / Report Section)
Evidence of flood risk permit / consent for proposed development, if applicable (from the EA (Flood Risk Activity Permit) or an Ordinary Watercourse Consent from the LLFA)		ORDINARY WATERCOURSE CONSENT REVIEWED
Outfall and discharge rate agreed in principle by water/sewer authority (if relevant)		THIS SURFACE PROVISIONAL 3-1-1/1 FOR



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