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Mid Sussex District Council
Flood Risk and Drainage Team
Oaklands Road
Haywards Heath
RH16 1SS

Our Ref: 10056-BHP-XX-XX-RP-D-1000
8 August 2025

Re: East Court, College Lane, East Grinstead. Application Reference DM/25/1324

I refer to the LLFA comments received in an email dated 5 August 2025. This letter seeks to address the comments.

LLFA Comments

The comments made in the LLFA response are as follows with responses below:

“Surface Water

It is proposed for surface water to be discharged from the site at an attenuated rate via cellular storage and orifice plates.

Based on the submitted information regarding infiltration, justification should be provided as to why infiltration SuDS are not viable at the site. Alternatively, an infiltration-based strategy should be provided based on the results of BRE365 methodology infiltration testing.”

The existing surface water drainage network has been maintained with connections added to accommodate the proposed development. The sewers flow through and discharge off-site at the northern boundary and not only convey the flows from the existing police station and car park but also the runoff of The Old Court House to the south of the site.

As such, the drainage system for the redevelopment must not impact the flows of The Old Court House as this would contradict the NPPF and PPG requirements to not increase flood risk elsewhere. The strategy proposed therefore maintains the routing and flows of the existing sewer, which traverse south to north through the site, but also provide a betterment to the system but restricting the flows of the development to 50% of its existing rate and therefore provides additional capacity within the current system. All restriction devices are located off the sewers main run, see Figure 1 below:

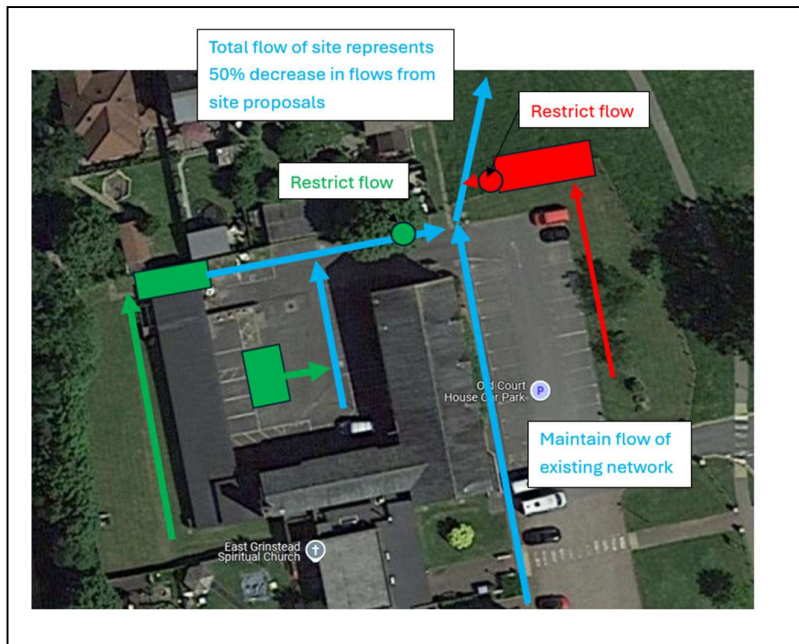


Figure 1 – Conceptual Diagram of Site Strategy.

The quantity of flows from The Old Court House are unknown and thus it would not be possible to determine if a system with a restricted outfall or an outfall subject to infiltration would be feasible. The quantity of storage as well as the function of an infiltration feature cannot be measured and in addition treatment of the existing flows of The Old Court House would need to be made to not cause ground pollution.

It is considered that it is not the responsibility of the developer to alter the arrangements and outfall of the existing sewer system for the adjacent Old Court House.

The most sustainable solution for the site is considered to be maintaining as much of the existing infrastructure as possible.

There is not sufficient evidence to suggest an infiltration strategy is feasible for the development as the current geotechnical report records low soakage rates at 1 location on the site's western boundary, where none of the infiltration features are proposed.

"The supplied calculations show the drainage system to flood for the 1:30+CC and 1:100+CC events. In line with policy, surface water can be stored in exceedance areas onsite for events between the 1:30+CC and 1:100+CC events. Free discharge off the site for the events between the 1:30+CC event and 1:100+CC event is unacceptable and does not meet with policy."

There is limited flooding in the 1:100+CC event, of up to 3.7m³ at its most extreme location. This extent of flooding is now shown in Drawing 10056-BHP-XX-XX-DR-C-2000 at the nodes flooding in the calculations. The extent of this is shown to an indicative 100mm depth.

It is considered the flooded water would flow downhill and be picked up again in the network downstream. To check the capacity of the network in this event, the calculations have been

re-run with flows directed to manhole EX-S15 where the water would be picked up by the gullies. The network does not flood in this instance and so the system has capacity to accommodate and convey these flows.

"Foul Water

The development will discharge foul water into the existing foul water sewer onsite. This is considered acceptable in principle.

The proposed calculations show the system flooding. Either updated calculations or clarification should be provided."

The flooding shown in the calculations is because of the limitations of the software used and this is a standard result. The software does not recognise an outfall or discharge point and as such the network conveys foul flows downstream to the "outfall" but does not consider a free-flowing discharge and so causes the network to back up and subsequently flood the lowest point on the network, which is at the end.

This comment is considered negligible.

Summary

I trust the above provides a suitable response to the points raised by the LLFA.

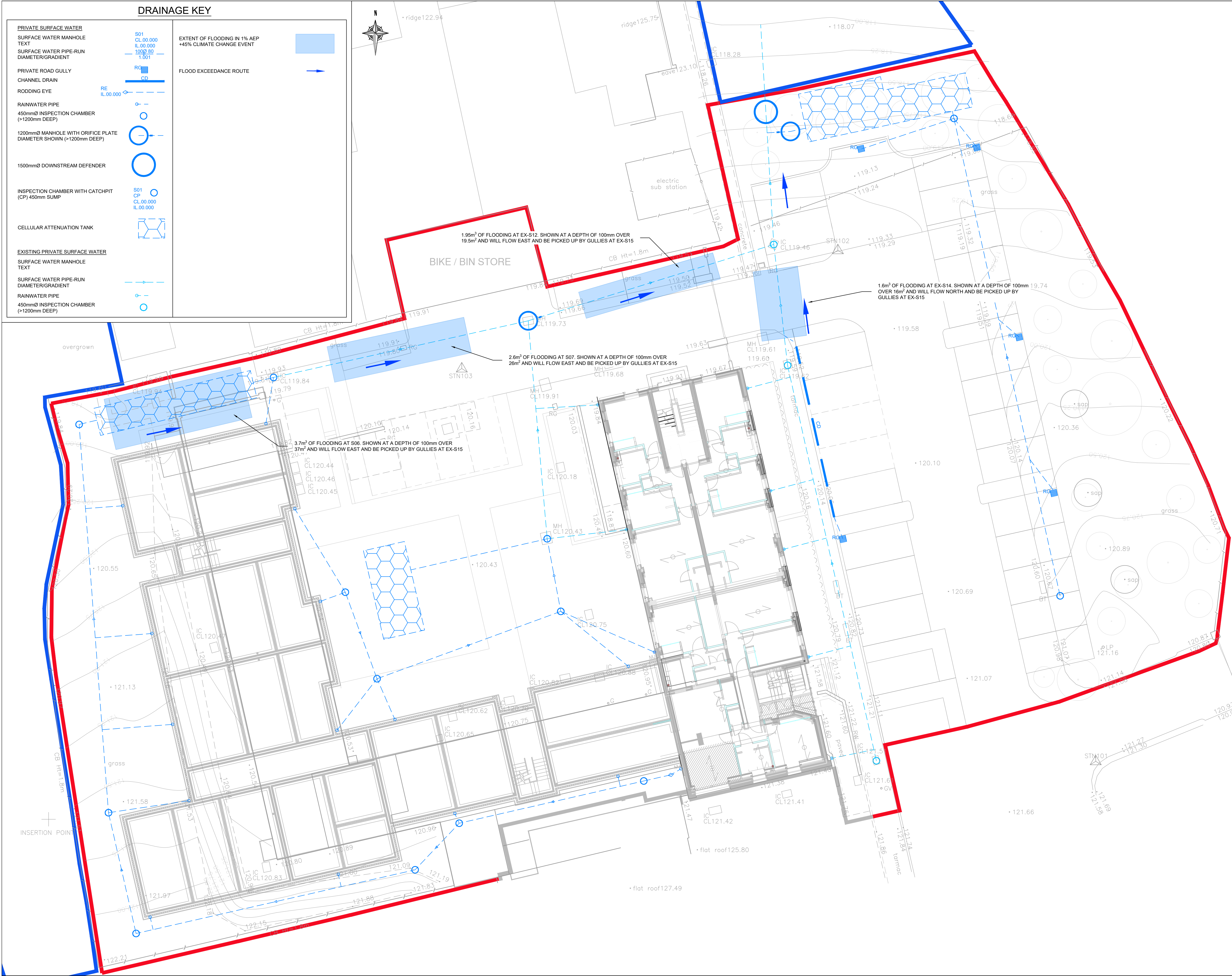
Yours sincerely,

Prepared by: Oliver Shaw
Civil Engineer

On behalf of Barter Hill Partnership Ltd

Encs

- Drawing 10056-BHP-XX-XX-DR-C-2000 Flood Exceedance
- Calculations 10056-BHP-003



DRAINAGE KEY

PRIVATE SURFACE WATER

SURFACE WATER MANHOLE
TEXT

SURFACE WATER PIPE-RUN
DIAMETER/GRADIENT

PRIVATE ROAD GULLY

CHANNEL DRAIN

RODDING EYE

RAINWATER PIPE

450mmØ INSPECTION CHAMBER
(>1200mm DEEP)

1200mmØ MANHOLE WITH ORIFICE PLATE
DIAMETER SHOWN (>1200mm DEEP)

1500mmØ DOWNSTREAM DEFENDER

INSPECTION CHAMBER WITH CATCHPIT
(CP) 450mm SUMP

CELLULAR ATTENUATION TANK

EXISTING PRIVATE SURFACE WATER

SURFACE WATER MANHOLE
TEXT

SURFACE WATER PIPE-RUN
DIAMETER/GRADIENT

RAINWATER PIPE

450mmØ INSPECTION CHAMBER
(>1200mm DEEP)

S01
CL 00.000
IL 00.000
1.001

RG
CD

RE
IL 00.000

CP
CL 00.000
IL 00.000

S01
CP
CL 00.000
IL 00.000

EXTENT OF FLOODING IN 1% AEP
+45% CLIMATE CHANGE EVENT

FLOOD EXCEEDANCE ROUTE

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CONSTRUCTION (DESIGN AND MANAGEMENT) REGULATIONS 2015

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 - DEEP DRAINAGE EXCAVATIONS. RISK OF FALLS, COLLAPSE OF TRENCH OR BATTER, CONFINED SPACES.
 - CONFLICT BETWEEN CONSTRUCTION WORKERS, VISITORS AND GENERAL PUBLIC WITH MOVING PLANT AND VEHICLES.
 - WORKS ADJACENT TO AND WITHIN PUBLIC ADOPTED HIGHWAY. CONFLICT BETWEEN CONSTRUCTION WORKERS/ACTIVITIES AND MOVING VEHICLES.
 - BANKING/ GRADING BETWEEN SITE BOUNDARY AND ADJACENT HIGHWAY STEEP IN PLACES. MEASURES TO BE IMPLEMENTED TO ENSURE WORKERS AND GENERAL PUBLIC ARE SAFE.
 - CARE TO BE TAKEN IN AREAS OF DENSE VEGETATION WHICH MAY CONTAIN HIDDEN OR UNIDENTIFIED RISKS.
- THE ABOVE NOTES REFER SPECIFICALLY TO THE INFORMATION SHOWN ON THIS DRAWING. REFER TO THE HEALTH AND SAFETY PLAN FOR FURTHER INFORMATION.

DRAWING FOR INFORMATION
NOT FOR CONSTRUCTION

P01	PLANNING ISSUE	OCS	JR	08.08.25
Rev.	Description	By	Chk.	Date

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& Civil Engineers**

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Meridian Business Park
Norwich NR7 0TA
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e: mail@barterhill.co.uk

Client
ZMQL LTD

Project
EAST COURT, EAST GRINSTEAD
RH19 3LT

Drawing Title
FLOOD EXCEEDANCE

Designed by OCS	Drawn by OCS	Checked by JR
Scale @ A1 1:125	Contact Office NORWICH	Date AUGUST 2025

Status PLANNING ISSUE	Revision P01	Job Ref 10056
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Drawing No
10056-BHP-XX-XX-DR-C-2000

Cad file
P:10056/Civils/BHP Drawings/10056-BHP-XX-XX-DR-C-2000

Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	100	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	1.000
CV	1.000	Preferred Cover Depth (m)	0.500
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	x
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
S01	0.005	5.00	120.658	450	539784.934	138653.399	0.778
S02	0.010	5.00	120.623	450	539771.737	138650.378	0.913
S03	0.005	5.00	120.518	450	539768.509	138647.372	0.863
S04	0.003	5.00	120.518	450	539748.556	138642.818	1.118
S05	0.020	5.00	120.518	450	539746.697	138650.969	1.223
S06	0.000		119.887	450	539744.598	138678.749	0.937
S07	0.003	5.00	119.840	450	539758.497	138682.111	1.100
S08	0.018	5.00	120.587	450	539763.498	138667.104	0.702
S09	0.012	5.00	120.578	450	539765.899	138660.556	0.783
S10	0.009	5.00	120.660	450	539779.036	138665.374	1.050
EX-S11	0.009	5.00	120.430	450	539778.065	138670.571	1.300
EX-S12	0.041	5.00	119.730	1200	539776.708	138686.166	1.500
EX-S13	0.018	5.00	121.510	450	539801.604	138654.684	1.100
EX-S14	0.025	5.00	119.620	450	539795.254	138682.977	1.100
EX-S15	0.000	5.00	119.228	450	539794.269	138691.625	1.368
S16	0.018	5.00	120.140	450	539814.743	138666.485	0.732
S17	0.024	5.00	119.228	450	539807.152	138700.642	1.709
EX-S18			118.879	1200	539795.439	138699.709	1.379
EX-S19			118.742	1500	539793.703	138701.108	1.361
DUMMY			118.015		539793.329	138707.360	1.361

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	S01	S02	13.538	0.600	119.880	119.710	0.170	79.6	150	5.20	50.0
1.001	S02	S03	4.411	0.600	119.710	119.655	0.055	80.2	150	5.27	50.0
1.002	S03	S04	20.466	0.600	119.655	119.400	0.255	80.3	150	5.57	50.0
1.003	S04	S05	8.360	0.600	119.400	119.295	0.105	79.6	150	5.69	50.0
1.004	S05	S06	27.860	0.600	119.295	118.950	0.345	80.8	150	6.11	50.0
1.005	S06	S07	14.300	0.600	118.950	118.740	0.210	68.1	150	6.30	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	1.127	19.9	0.9	0.628	0.763	0.005	0.0	22	0.573
1.001	1.123	19.8	2.7	0.763	0.713	0.015	0.0	37	0.786
1.002	1.123	19.8	3.6	0.713	0.968	0.020	0.0	43	0.853
1.003	1.127	19.9	4.2	0.968	1.073	0.023	0.0	47	0.894
1.004	1.119	19.8	7.8	1.073	0.787	0.043	0.0	65	1.053
1.005	1.220	21.6	7.8	0.787	0.950	0.043	0.0	62	1.124

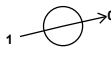
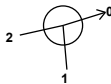
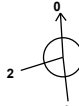
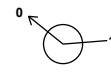
Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
2.000	6.974	77.5	150	Circular	120.587	119.885	0.552	120.578	119.795	0.633
2.001	13.993	75.6	150	Circular	120.578	119.795	0.633	120.660	119.610	0.900
2.002	5.287	11.0	150	Circular	120.660	119.610	0.900	120.430	119.130	1.150
2.003	15.654	18.2	110	Circular	120.430	119.130	1.190	119.730	118.270	1.350
1.007	18.390	49.7	150	Circular	119.730	118.230	1.350	119.228	117.860	1.218
3.000	28.997	15.3	110	Circular	121.510	120.410	0.990	119.620	118.520	0.990
3.001	8.704	14.0	110	Circular	119.620	118.520	0.990	119.228	117.900	1.218
1.008	9.500	21.7	150	Circular	119.228	117.860	1.218	118.742	117.423	1.169
4.000	34.991	18.5	150	Circular	120.140	119.408	0.582	119.228	117.519	1.559
4.001	1.000	52.6	150	Circular	119.228	117.519	1.559	118.879	117.500	1.229
4.002	1.650	21.4	150	Circular	118.879	117.500	1.229	118.742	117.423	1.169
1.009	6.263	8.6	150	Circular	118.742	117.381	1.211	118.015	116.654	1.211

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
2.000	S08	450	Manhole	Adoptable	S09	450	Manhole	Adoptable
2.001	S09	450	Manhole	Adoptable	S10	450	Manhole	Adoptable
2.002	S10	450	Manhole	Adoptable	EX-S11	450	Manhole	Adoptable
2.003	EX-S11	450	Manhole	Adoptable	EX-S12	1200	Manhole	Adoptable
1.007	EX-S12	1200	Manhole	Adoptable	EX-S15	450	Manhole	Adoptable
3.000	EX-S13	450	Manhole	Adoptable	EX-S14	450	Manhole	Adoptable
3.001	EX-S14	450	Manhole	Adoptable	EX-S15	450	Manhole	Adoptable
1.008	EX-S15	450	Manhole	Adoptable	EX-S19	1500	Manhole	Adoptable
4.000	S16	450	Manhole	Adoptable	S17	450	Manhole	Adoptable
4.001	S17	450	Manhole	Adoptable	EX-S18	1200	Manhole	Adoptable
4.002	EX-S18	1200	Manhole	Adoptable	EX-S19	1500	Manhole	Adoptable
1.009	EX-S19	1500	Manhole	Adoptable	DUMMY		Junction	

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	
S01	539784.934	138653.399	120.658	0.778	450		0	1.000	119.880	150
S02	539771.737	138650.378	120.623	0.913	450		1	1.000	119.710	150
S03	539768.509	138647.372	120.518	0.863	450		0	1.001	119.710	150
S04	539748.556	138642.818	120.518	1.118	450		1	1.001	119.655	150
S05	539746.697	138650.969	120.518	1.223	450		0	1.002	119.655	150
							1	1.002	119.400	150
							0	1.003	119.400	150
							1	1.003	119.295	150
							0	1.004	119.295	150

	Barter Hill Partnership Ltd	File: 10056-Model Surface Final (Flooded Routes) Network: Oliver Shaw 08/08/2025	Page 4 Flooded Routes 10056-BHP-003							
Manhole Schedule										
Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	
S06	539744.598	138678.749	119.887	0.937	450		1	1.004	118.950	150
S07	539758.497	138682.111	119.840	1.100	450		0	1.005	118.950	150
						1	1.005	118.740	150	
S08	539763.498	138667.104	120.587	0.702	450		0	1.006	118.740	110
						0	2.000	119.885	150	
S09	539765.899	138660.556	120.578	0.783	450		1	2.000	119.795	150
						0	2.001	119.795	150	
S10	539779.036	138665.374	120.660	1.050	450		1	2.001	119.610	150
						0	2.002	119.610	150	
EX-S11	539778.065	138670.571	120.430	1.300	450		1	2.002	119.130	150
						0	2.003	119.130	110	
EX-S12	539776.708	138686.166	119.730	1.500	1200		1	2.003	118.270	110
						2	1.006	118.270	110	
EX-S13	539801.604	138654.684	121.510	1.100	450		0	1.007	118.230	150
						0	3.000	120.410	110	
EX-S14	539795.254	138682.977	119.620	1.100	450		1	3.000	118.520	110
						0	3.001	118.520	110	
EX-S15	539794.269	138691.625	119.228	1.368	450		1	3.001	117.900	110
						2	1.007	117.860	150	
S16	539814.743	138666.485	120.140	0.732	450		0	1.008	117.860	150
						0	4.000	119.408	150	
S17	539807.152	138700.642	119.228	1.709	450		1	4.000	117.519	150
						0	4.001	117.519	150	
EX-S18	539795.439	138699.709	118.879	1.379	1200		1	4.001	117.500	150
						0	4.002	117.500	150	

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barter hill	Barter Hill Partnership Ltd	File: 10056-Model Surface Final (Flooded Routes)				Page 5																																																																	
		Network: Oliver Shaw 08/08/2025				Flooded Routes 10056-BHP-003																																																																	
Manhole Schedule <table><tr><th>Node</th><th>Easting (m)</th><th>Northing (m)</th><th>CL (m)</th><th>Depth (m)</th><th>Dia (mm)</th><th>Connections</th><th>Link</th><th>IL (m)</th><th>Dia (mm)</th></tr><tr><td>EX-S19</td><td>539793.703</td><td>138701.108</td><td>118.742</td><td>1.361</td><td>1500</td><td rowspan="3"></td><td>1</td><td>4.002</td><td>117.423</td><td>150</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td>2</td><td>1.008</td><td>117.423</td><td>150</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td>0</td><td>1.009</td><td>117.381</td><td>150</td></tr><tr><td>DUMMY</td><td>539793.329</td><td>138707.360</td><td>118.015</td><td>1.361</td><td></td><td rowspan="2"></td><td>1</td><td>1.009</td><td>116.654</td><td>150</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr></table>										Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	EX-S19	539793.703	138701.108	118.742	1.361	1500		1	4.002	117.423	150							2	1.008	117.423	150							0	1.009	117.381	150	DUMMY	539793.329	138707.360	118.015	1.361			1	1.009	116.654	150										
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Simulation Settings <table><tr><td>Rainfall Methodology</td><td>FEH-22</td><td>Analysis Speed</td><td>Normal</td><td>Starting Level (m)</td><td></td></tr><tr><td>Rainfall Events</td><td>Singular</td><td>Skip Steady State</td><td>x</td><td>Check Discharge Rate(s)</td><td>x</td></tr><tr><td>Summer CV</td><td>1.000</td><td>Drain Down Time (mins)</td><td>1440</td><td>Check Discharge Volume</td><td>x</td></tr><tr><td>Winter CV</td><td>1.000</td><td>Additional Storage (m³/ha)</td><td>0.0</td><td></td><td></td></tr></table>										Rainfall Methodology	FEH-22	Analysis Speed	Normal	Starting Level (m)		Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s)	x	Summer CV	1.000	Drain Down Time (mins)	1440	Check Discharge Volume	x	Winter CV	1.000	Additional Storage (m³/ha)	0.0																																								
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Winter CV	1.000	Additional Storage (m³/ha)	0.0																																																																				
Storm Durations <table><tr><td>15</td><td>60</td><td>180</td><td>360</td><td>600</td><td>960</td><td>2160</td><td>4320</td><td>7200</td><td>10080</td></tr><tr><td>30</td><td>120</td><td>240</td><td>480</td><td>720</td><td>1440</td><td>2880</td><td>5760</td><td>8640</td><td></td></tr></table>										15	60	180	360	600	960	2160	4320	7200	10080	30	120	240	480	720	1440	2880	5760	8640																																											
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Node S06 Offline Orifice Control <table><tr><td>Flap Valve</td><td>x</td><td>Invert Level (m)</td><td>119.800</td><td>Discharge Coefficient</td><td>0.600</td></tr><tr><td>Loop to Node</td><td>EX-S15</td><td>Diameter (m)</td><td>10.000</td><td></td><td></td></tr></table>										Flap Valve	x	Invert Level (m)	119.800	Discharge Coefficient	0.600	Loop to Node	EX-S15	Diameter (m)	10.000																																																				
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Node EX-S14 Offline Orifice Control <table><tr><td>Flap Valve</td><td>x</td><td>Invert Level (m)</td><td>119.600</td><td>Discharge Coefficient</td><td>0.600</td></tr><tr><td>Loop to Node</td><td>EX-S15</td><td>Diameter (m)</td><td>10.000</td><td></td><td></td></tr></table>										Flap Valve	x	Invert Level (m)	119.600	Discharge Coefficient	0.600	Loop to Node	EX-S15	Diameter (m)	10.000																																																				
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Node EX-S15 Offline Orifice Control

Flap Valve	x	Invert Level (m)	119.200	Discharge Coefficient	0.600
Loop to Node	EX-S19	Diameter (m)	10.000		

Node S06 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	5.0	Invert Level (m)	118.950
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.95	Time to half empty (mins)	14

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	23.0	0.0	0.400	23.0	0.0	0.401	0.0	0.0

Node EX-S11 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	5.0	Invert Level (m)	119.200
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.95	Time to half empty (mins)	16

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	19.5	0.0	0.800	19.5	0.0	0.801	0.0	0.0

Node EX-S18 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	5.0	Invert Level (m)	117.500
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.95	Time to half empty (mins)	328

Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	36.0	0.0	0.800	36.0	0.0	0.801	0.0	0.0

Results for 100 year +45% CC Critical Storm Duration. Lowest mass balance: 99.76%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	S01	21	120.419	0.539	4.1	0.0857	0.0000	FLOOD RISK
30 minute summer	S02	21	120.415	0.705	11.0	0.1121	0.0000	FLOOD RISK
30 minute summer	S03	21	120.403	0.748	14.1	0.1189	0.0000	FLOOD RISK
30 minute summer	S04	21	120.326	0.925	12.9	0.1472	0.0000	FLOOD RISK
30 minute summer	S05	20	120.280	0.985	25.7	0.1566	0.0000	FLOOD RISK
15 minute summer	S06	12	119.812	0.862	35.7	8.8880	0.0000	FLOOD RISK
60 minute summer	S07	33	119.817	1.077	10.7	0.1712	0.0000	FLOOD RISK
15 minute summer	S08	12	120.241	0.356	15.9	0.0567	0.0000	SURCHARGED
15 minute summer	S09	12	120.191	0.396	25.6	0.0629	0.0000	SURCHARGED
30 minute summer	S10	21	119.992	0.382	29.4	0.0608	0.0000	SURCHARGED
30 minute summer	EX-S11	24	119.962	0.832	37.6	14.2493	0.0000	SURCHARGED
15 minute summer	EX-S12	12	119.713	1.483	37.0	1.6774	0.0000	FLOOD RISK
15 minute summer	EX-S13	11	120.710	0.300	15.9	0.0478	0.0000	SURCHARGED
30 minute summer	EX-S14	19	119.611	1.091	34.5	0.1735	0.0000	FLOOD RISK
30 minute summer	EX-S15	20	119.217	1.357	82.6	0.2158	0.0000	FLOOD RISK
15 minute summer	S16	10	119.472	0.064	15.9	0.0102	0.0000	OK
360 minute summer	S17	264	118.297	0.778	7.9	0.1237	0.0000	SURCHARGED
360 minute summer	EX-S18	264	118.297	0.797	7.6	28.1558	0.0000	SURCHARGED
30 minute summer	EX-S19	20	118.257	0.876	82.8	1.5471	0.0000	SURCHARGED
15 minute summer	DUMMY	15	116.806	0.152	70.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	S01	1.000	S02	3.6	0.455	0.181	0.2383	
30 minute summer	S02	1.001	S03	10.4	0.861	0.526	0.0777	
30 minute summer	S03	1.002	S04	10.9	0.991	0.548	0.3603	
30 minute summer	S04	1.003	S05	12.3	0.770	0.618	0.1472	
30 minute summer	S05	1.004	S06	22.9	1.301	1.158	0.4905	
15 minute summer	S06	1.005	S07	10.5	0.890	0.488	0.2517	
15 minute summer	S06	Orifice	EX-S15	19.8				4.9
60 minute summer	S07	1.006	EX-S12	11.0	1.164	0.887	0.1766	
15 minute summer	S08	2.000	S09	15.0	0.855	0.745	0.1228	
15 minute summer	S09	2.001	S10	24.9	1.468	1.220	0.2463	
30 minute summer	S10	2.002	EX-S11	28.6	1.681	0.530	0.0931	
30 minute summer	EX-S11	2.003	EX-S12	12.9	1.358	0.699	0.1482	
15 minute summer	EX-S12	Orifice	EX-S15	23.7				
15 minute summer	EX-S12	Orifice	EX-S15	22.3				3.0
15 minute summer	EX-S13	3.000	EX-S14	14.2	1.544	0.709	0.2745	
30 minute summer	EX-S14	3.001	EX-S15	24.6	2.598	1.173	0.0824	
30 minute summer	EX-S14	Orifice	EX-S15	17.6				3.4
30 minute summer	EX-S15	1.008	EX-S19	58.1	3.300	1.515	0.1672	
30 minute summer	EX-S15	Orifice	EX-S19	34.0				4.7
15 minute summer	S16	4.000	S17	15.8	1.096	0.381	0.4340	
360 minute summer	S17	4.001	EX-S18	7.6	0.834	0.310	0.0176	
360 minute summer	EX-S18	Orifice	EX-S19	1.2				
30 minute summer	EX-S19	1.009	DUMMY	76.6	4.353	1.256	0.1102	113.5